

LIM Report

This information has been supplied by the Vendor or the Vendor's agents. Accordingly, Optimize Realty Limited cannot guarantee its accuracy and reliability.

All intending Purchasers are advised to conduct their own due diligence investigation into the same. To the maximum extent permitted by law, Optimize Realty Limited do not accept any responsibility to any person for the accuracy of the information herein.

Harcourts Whangarei



Private Bag 9093. Whangarel 0148, New Zealand 2 +64 9 430 4200 | 0800 WDC INFO | 0800 932 463 // +64 9 438 7632 7 mailroom@wdc.govt.nz 3958/1946.govt.nz

LAND INFORMATION MEMORANDUM NO: LM2300968 Received: 22 Aug 2023 Issued: 30 Aug 2023 Section 44A, Local Government Official Information And Meetings Act 1987

APPLICANT

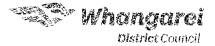
CTA Guy 171A Tara Road RD 2 Kaiwaka 0573

SITE INFORMATION

Property ID: 172212 Street Address: 41 Kaurinui Crescent One Tree Point 0118 Legal Description: LOT 407 DP 564989

This is a Land Information Memorandum only.

Full payment has been made for this Land Information Memorandum.



1: **PROPERTY DETAILS.**

- Location Map
- Deposited Plan: DP 564989
- Record of Title: 1009463

This property is subject to a Consent Notice, information attached.

- Interest Number 12201143.6. Dated 03/11/2021
- 2: INFORMATION IDENTIFYING EACH (IF ANY) SPECIAL FEATURE OR CHARACTERISTIC OF THE LAND CONCERNED, INCLUDING BUT NOT LIMITED TO POTENTIAL EROSION, AVULSION, FALLING DEBRIS, SUBSIDENCE, SLIPPAGE, ALLUVION, OR INUNDATION, OR LIKELY PRESENCE OF HAZARDOUS CONTAMINANTS, BEING A FEATURE OR CHARACTERISTIC THAT IS KNOWN TO THE WHANGAREI DISTRICT COUNCIL.

Whangarei District Council holds indicative information on land stability hazard for Whangarei. Information on land stability, including an interactive web tool, can be found on the WDC website. The Whangarei District Council may require site-specific investigations before granting future subdivision or building consent for the property, the level of investigation or assessment would depend on the level of stability risk of the area the property is in.

See map attached indicating this property is located within low zone and refer: https://www.wdc.govt.nz/Services/My-property-and-rates/Natural-hazards

Whangarei District Council notified Plan Change 1 - Natural Hazards (PC1) on the 31st of May 2023. The Plan Change proposes to replace the existing Natural Hazards chapter in the District Plan Operative in Part 2022 with a new Natural Hazards chapter and new rules for subdivision and land use in hazard prone areas. For more information on the proposed plan change please visit: <u>https://www.wdc.govt.nz/Services/Planning/District-Plan_changes/Current.plan.</u>

<u>changes</u>

This property has been identified as having information available under Stormwater Catchment and Flood Management.

Any overland flow path shown provides an indicative understanding of routes where surface stormwater may flow during rainfall events.

Any depression storage area/surface depression ponding area shown provides an indicative extent of ponding that may form occur if the outlet to the ponding area is blocked or where the outlet capacity is exceeded eg a pipe is either blocked or the design capacity of the pipe is exceeded during a rainfall event.

Refer Pipeline Assets map attached and for further information please contact the Waste and Drainage Department on 09 430 4200



This property is in an area that has been identified to contain:

Acid Sulphate Soil Risk

A copy of the Opus Acid Sulphate Soil guidance document dated August 2015 can be found on the Whangarei District Council website.

For information refer:

https://www.wdc.govt.nz/Council/Council-documents/Policies/Acid-Sulphate-Soil-Planning-Policy

Whangarei District Council holds information on the liquefaction vulnerability of the district. The site is located within an area classified as Liquefaction vulnerability category is possible.

The report was prepared by Tonkin & Taylor Ltd to provide WDC with a district wide liquefaction vulnerability assessment to help inform spatial planning and assessment of landuse, subdivision and building consents.

To view the report and access maps please use the following link: https://www.wdc.govt.nz/Services/Mv-property-and-rates/Natural-hazards

Please note: To view the liquefaction layer your map scale must be greater than 1:5000.

3: INFORMATION ON COUNCIL AND PRIVATE UTILITY (SEWERAGE, WATER & STORMWATER) SERVICES.

Information relating to Council Utility Services for this property is attached.

Pipeline Assets Map

As-Built, House Connection and/or Drainage Plan for this property from the building file is attached.

As Built Services Plan – From BC2200495

Pursuant to Section 51 of the Building Act 2004 and Section 451 of the Local Government Act 1974, any future building work that encroaches upon any Council Pipe or Utility must obtain written consent from the Waste & Drainage and/or Water Services Manager/s prior to works commencing.

For information refer:

https://www.wdc.govt.nz/Council/Council-documents/Policies/Building-Over-Public-Sewers-Policy

This property is within the area of benefit of a pressure sewer system. See map attached and for the Homeowners guide and key requirements for maintaining the system please refer:

https://www.wdc.govf.nz/Services/Water-sorvices/Wastewater/Pressure-Sewer-Systems

4: INFORMATION RELATING TO VALUATION, LAND, AND WATER RATES. INFORMATION FROM WHANGAREI DISTRICT COUNCIL RECORDS.

Information on Valuation, Rates and Water Meter location (if applicable) for the current financial year, is attached.



Outstanding water balance as at today's date is \$0.00. A final reading of the water meter will be required.

5: INFORMATION CONCERNING ANY PERMIT, CONSENT, CERTIFICATE, NOTICE ORDER, OR REQUISITION AFFECTING THE LAND OR ANY BUILDING ON THE LAND PREVIOUSLY ISSUED BY THE WHANGAREI DISTRICT COUNCIL OR BUILDING CERTIFIER (WHETHER UNDER THE BUILDING ACT 1991 AND/OR 2004 OR ANY OTHER ACT).

Copy of a Building Consent and Code Compliance Certificate issued for this property is attached.

 BC2200495 – New Dwelling Building Consent – Issued 10/06/2022 Code Compliance Certificate – Issued 16/01/2023

Copy of Applications (e.g. Vehicle Crossing Permit and/or Public Utility Service) for this property are attached.

- Vehicle Crossing Permit VC220104 – – Issued 02/06/2022
- Pressurised Sewer, Relocation of Water Connection, and Installation of Water Meter
 PU221209 – Approved 15/06/2022

6: INFORMATION RELATING TO THE USE TO WHICH THE LAND MAY BE PUT AND ANY CONDITIONS ATTACHED TO THAT USE.

This property is located in a General Residential Zone. See map attached and refer to Part 3: Area Specific Matters - Chapters - Residential zones. https://www.wdc.govt.nz/Services/Property/Planning/Operative-District-Plan

7: INFORMATION WHICH IN TERMS OF ANY OTHER ACT HAS BEEN NOTIFIED TO THE WHANGAREI DISTRICT COUNCIL BY ANY STATUTORY ORGANISATION HAVING THE POWER TO CLASSIFY LAND OR BUILDINGS FOR ANY PURPOSE.

This property is known to contain/or is in the vicinity of Archaeological site/s, information attached.

- Q07/1419 Midden/Oven
 Q07/1421 Midden/Oven
- Q07/1420 Midden/Oven
 Q07/1422 Midden /Oven

For further information contact the Area Archaeologist at Heritage New Zealand, Northland Area Office on ph. 09 407 0470 or infonorthland@heritage.org.nz



8: OTHER INFORMATION CONCERNING THE LAND AS WHANGAREI DISTRICT COUNCIL CONSIDERS, AT COUNCILS DISCRETION, TO BE RELEVANT.

Whangarei District Council recommends that all Whangarei District residents visit the Northland Regional Council website, <u>https://www.nrc.govt.nz/forinformation on Civil</u> Defence hazard response. This information includes Tsunami evacuation zones, maps and community response plans for flooding and extreme weather events etc.

Copies of site plan, floor plan and elevations are attached for your information.

A copy of a Ground Conditions Assessment Report by Wilton Joubert approved 09/06/2022 from BC2200495 is attached for your information.

9: INFORMATION RELATING TO ANY UTILITY SERVICE OTHER THAN COUNCILS SUCH AS TELEPHONE, ELECTRICITY, GAS AND REGIONAL COUNCIL WILL NEED TO BE OBTAINED FROM THE RELEVANT UTILITY OPERATOR.

Further information may be available from other authorities; Northpower; Spark; Vector Limited; etc.

DISCLAIMER

Land Information Memoranda (LIM) are prepared under the provisions of Section 44A of the Local Government Official Information and Meetings Act 1987. An inspection of the land or building(s) has not been completed for the purposes of preparing the LIM. It has been compiled from the records held by Whangarei District Council. The information contained in the LIM is correct at the date of issue.

A LIM is prepared for the use of the applicant and may not be able to be relied on by other parties.

Advice from an independent professional such as a lawyer or property advisor should be sought regarding the contents of this LIM.

Additional information regarding the land or buildings (such as resource consents and other permissions and restrictions) not contained in this LIM may be held by Northland Regional Council. For further information contact Northland Regional Council on (09) 470 1200, 0800 002 004 or www.nrc.govt.nz.

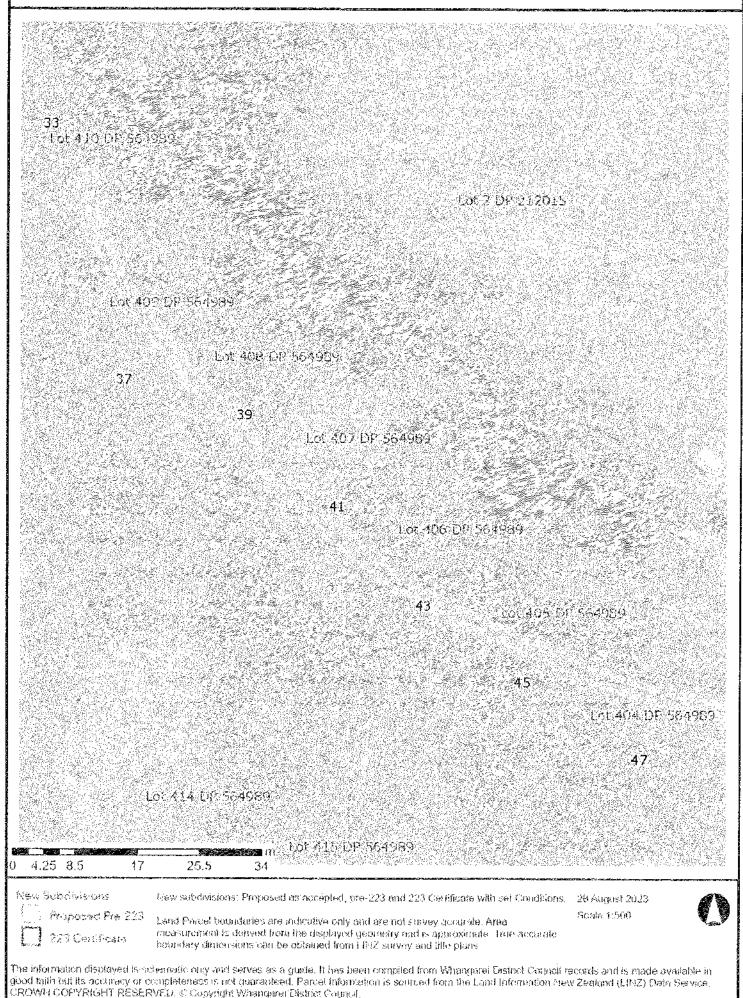
A LIM is not a suitable search of Council's records for the purposes of the National Environmental Standards (NES) for soil contamination of a potentially contaminated site.

Signed for and on behalf of Council:

Elle Swanson Property Assessment Officer

Property Map









Title Plan - DP 564989

Survey Number Surveyor Reference	DP 564989 14333- 004 - Stage 4 The	Landings		
Surveyor	Donald Alistair Turner			
Survey Firm Surveyar Boolaration	CCL 2015 Ltd I Donald Alistair Turner	being a licensed cadastral surveyor, co	ertify that:	
our veyor ibeciar atton	(a) this dataset provided	by me and its related survey are accur	ate, correct and in	accordance with th
	Cadastral Survey Act 200	2 and the Rules for Cadastral Survey	2010, and	
	 (b)the survey was underta Declared on 21 Feb 2022 	aken by me or under my personal dire 07:51 AM	ction.	
Survey Details				· •••• · · · · · · · · · · · · · · · ·
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		ing a subdivision of Lot 1000 DP 5489	998 and Lot 2 DF	00100
Status	Deposited	Course Chara	Class A	
Land District	North Anekland	Survey Class		
Submitted Date	21/02/2022	Survey Approval I	21/02/2022	
		Deposit Date	21/0///2022	
Territorial Authoritie				· · · · · · · · · · · · · · · · · · ·
Whangarei District				
Comprised In				
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RT 942323 RT 237117 Created Parcels Parcels Area J Deposited Plan			Area 0.0707 Ha	RT Reference 1009366
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Area

0.0651 Ha

0.0771 Ha

0.0620 Ha

RT Reference

1009453

1009454

1009455

Title Plan - DP 564989

Created Parcels

Parcels

Lot 397 Deposited Plan 564989 Lot 398 Deposited Plan 564989 Lot 399 Deposited Plan 564989 Lot 400 Deposited Plan 564989 Lot 401 Deposited Plan 564989 Lot 402 Deposited Plan 564989 Lot 403 Deposited Plan 564989 Lot 404 Deposited Plan 564989 Lot 405 Deposited Plan 564989 Lot 406 Deposited Plan 564989 Lot 407 Deposited Plan 564989 Lot 408 Deposited Plan 564989 Lot 409 Deposited Plan 564989 Lot 410 Deposited Plau 564989 Lot 411 Deposited Plan 564989 Lot 412 Deposited Plan 564989 Lot 413 Deposited Plan 564989 Lot 414 Deposited Plan 564989 Lot 415 Deposited Plan 564989 Lot 416 Deposited Plan 564989 Lot 417 Deposited Plan 564989 Lot 418 Deposited Plan 564989 Lot 419 Deposited Plan 564989 Lot 420 Deposited Plan 564989 Lot 421 Deposited Plan 564989 Lot 422 Deposited Plan 564989 Lot 423 Deposited Plan 564989 Lot 424 Deposited Plan 564989 Lot 425 Deposited Plan 564989 Lot 426 Deposited Plan 564989 Lot 427 Deposited Plan 564989 Lot 428 Deposited Plan 564989 Lot 265 Deposited Plan 564989 Lot 266 Deposited Plan 564989 Lot 267 Deposited Plan 564989 Lot 268 Deposited Plan 564989 Lot 269 Deposited Plan 564989 Lot 270 Deposited Plan 56/989 Lot 319 Deposited Plan 564989 Lot 713 Deposited Plan 564989 Lot 551 Deposited Plan 564989

Lot 712 Deposited Plan 564989 Lot 709 Deposited Plan 564989

Fee Simple Title 0.0696 Ha 1009456 Fee Simple Title 0.0684 Ha 1009457 Fee Simple Title 0.0639 Ha 1009458 Fee Simple Title 0.0639 Ha 1009459 Fee Simple Title 0.0643 Ha 1009460 Fee Simple Title 0.0682 Ha 1009461 Fee Simple Title 0.0671 Ha 1009462 Fee Simple Title 0.0597 Ha 1009463 Fee Simple Title 0.055014a 1009464 Fee Simple Title 0.0660 Ha 1009465 Fee Simple Title 0.0601 Ha 1009466 Fee Simple Title 0.0602 Ha 1009467 Fee Simple Title 0.0596 Ha 1009468 Fee Simple Title 0.0609 Ha 1009469 Fee Simple Title 0.0577 Ha 1009371 Fee Simple Title 0.0627 Ha 1009372 Fee Simple Title 0.0549 Ha 1009373 Fee Simple Title 0.0589 Ha 1009374 Fee Simple Title 0.0551 Ha 1009375 Fee Simple Title 0.0589 Ha 1009376 Fee Simple Title 0.055114a 1009377 Fee Simple Title 0.0590 Ha 1009378 Fee Simple Title 0.0550 Ha 1009379 Fee Simple Title 0.0590 Ha 1009380 Fee Simple Title 0.0589 Ha 1009381 Fee Simple Title 0.0631 Ha 1009382 Fee Simple Title 0.0585 Ha 1009383 Fee Simple Title 0.0604 Ha 1009384 Fee Simple Title 1009385 0.0651 Ha Fee Simple Title 0.0721 Ha 1009415 Fee Simple Title 0.0604 Ha 1009416 Fee Simple Title 0.0600 Ha 1009417 Fee Simple Title 0.0650 Ha 1009418 Fee Simple Title 0.0652 Ha 1009386 Fee Simple Title 0.0602 Ha 1009387 Fee Simple Title 0.0600 Ha 1009431 Fee Simple Title 0.0998 Ha Multiple Vesting on Deposit for 1.1628 Ha 1009439 Local Purpose Reserve Fcc Simple Title 0.0118 Ha Multiple Fee Simple Title 0.0282 Ha Multiple

DP 564989 - Title Plan

Generalad on 22/03/2022 2:12pm

Parcel Intent

Fee Simple Title

Fee Simple Title

Fee Simple Title

Page 2 of 19





Title Plan - DP 564989

Created Parcels

Parcels Lot 604 Deposited Plan 564989

Lot 2000 Deposited Plan 564989 Area A Deposited Plan 564989 Area B Deposited Plan 564989 Area C Deposited Plan 564989 Area D Deposited Plan 564989 Area E Deposited Plan 564989 Area F Deposited Plan 564989 Area G Deposited Plan 564989 Total Area

Parcel Intent
Vesting on Deposit for
Local Purpose Reserve
Road
Fee Simple Title
Easement

 Area
 RT Reference

 0.1014 Ha
 1009388

 1.5550 Ha

26.6464 Ha 1009470

33.0640 Ha

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Schedule / Memorandum

North Auckland

DP 564989

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Territorial Authority (the Council) Whangarei District Council

Memorandum of Easements (Pursuant to s243 Resource Management Act 1991)				
Purpose	Shown	Servient Tenement (Burdened land)	Dominant Tenement (Benefited land)	
	Lot 709	Lot 709	Lots 380, 381, 386 & 387 Hereon	
Right to convey water	Lot 712	Lot 712	Lots 410 & 412 Hereon	
794-344-693-784-46-46-4-4	Lot 713	Lot 713	Lots 415, 417, 419, 421, 423, 424 & 425 Hereon	

Memorandum of Easements in Gross (Pursuant to s243 Resource Management Act 1991)				
Purpose	Shown	Servient Tenement	Grantee	
Right to drain water &	Lot 709	Lot 709		
sewage	Lot 712	Lot 712	Whangarei District Council	
	Lot 713	Lot 713		
Right to convey	Lot 709	Lot 709		
electricity & telecommunications	Lot 712	l.ot 712	Northpower Limited & Northpower Fibre Limited	
	Lot 713	Lot 713		
	A			
Right to convey water	В	Lot 2000	Whangarei District Council	
Part. (A		· · (h	
Right to drain sewage	8	Lot 2000	Whangarei District Council	
Right of Way	B, C, D, E, F, G	Lot 2000	Whangarei District council	

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North Auckland	DP 564989

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Territorial Authority (the Council) Whangarei District Council

Schedule of Existing Easements in Gross				
Purpose	Shown	Servient Tenement (Burdened land)	Creating Document Reference	
Right of Way	l.	Lot 2000	EI 11747376.19	
Right to convey electricity, telecommunications & water	J	Lot 2000	El 11747376.19	
Right to drain water & sewage	J	Lot 2000	El 11747376.19	

Amalgamation Conditions

The following amalgamations are required as part of this subdivision -- LINZ Ref. 1693818

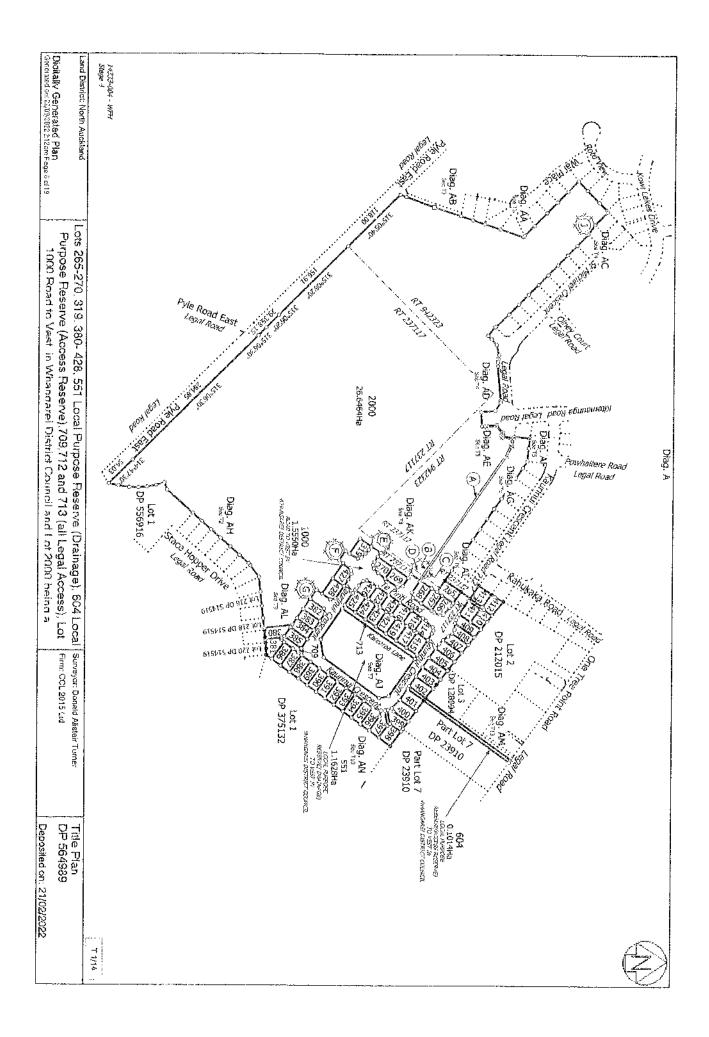
That Lot 709 hereon (Legal Access) be held as to four undivided one-fourth shares by the owners of Lots 380, 381, 386 and 387 hereon as tenants in common in the said shares and that Individual Computer Registers be issued in accordance herewith. (LINZ Ref. 1693818)

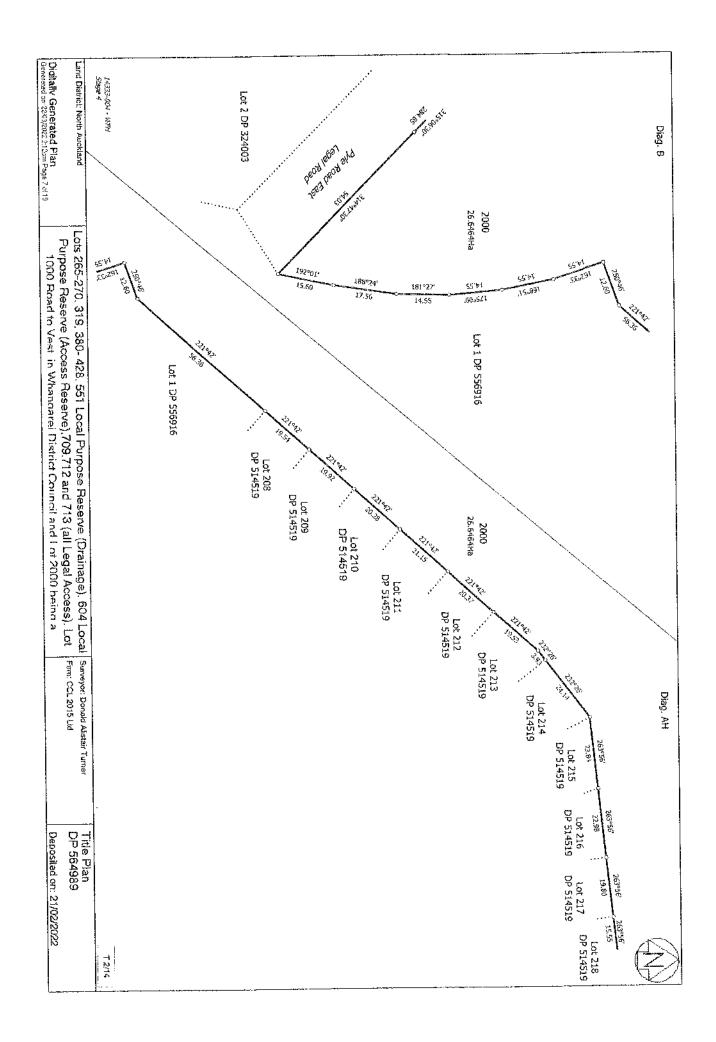
That Lot 712 hereon (Legal Access) be held as to two undivided one-half shares by the owners of Lots 410 and 412 hereon as tenants in common in the said shares and that Individual Computer Registers be issued in accordance herewith. (LINZ Ref. 1693818)

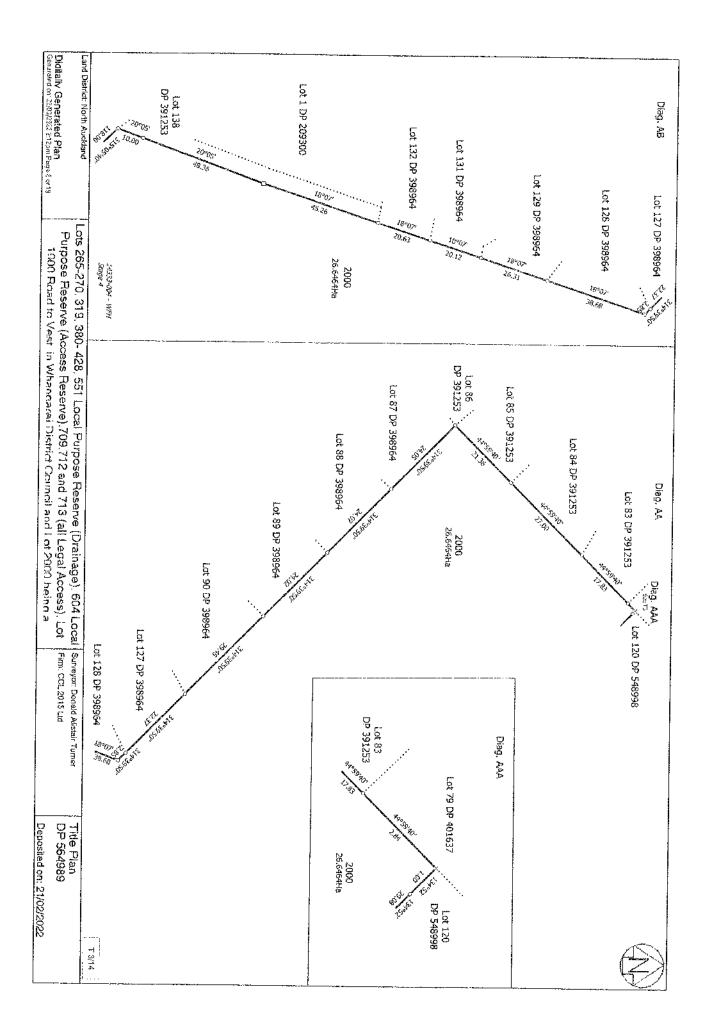
That Lot 713 hereon (Legal Access) be held as to seven undivided one-seventh shares by the owners of Lots 415, 417, 419, 421, 423, 424 and 425 hereon as tenants in common in the said shares and that Individual Computer Registers be issued in accordance herewith. (LINZ Ref. 1693818)

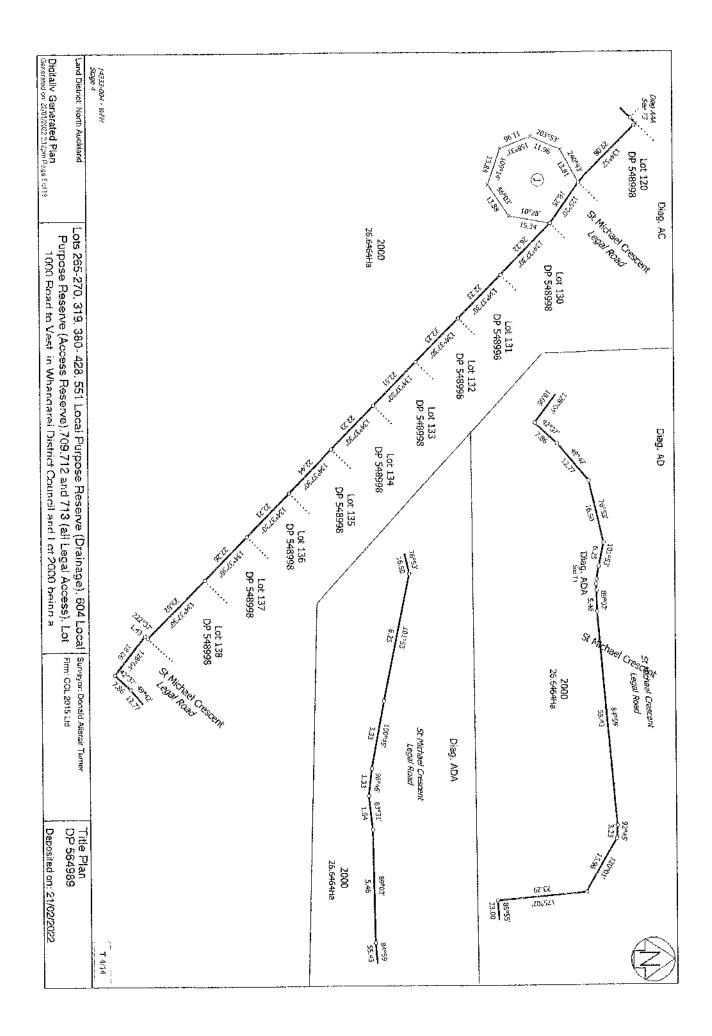
Notes

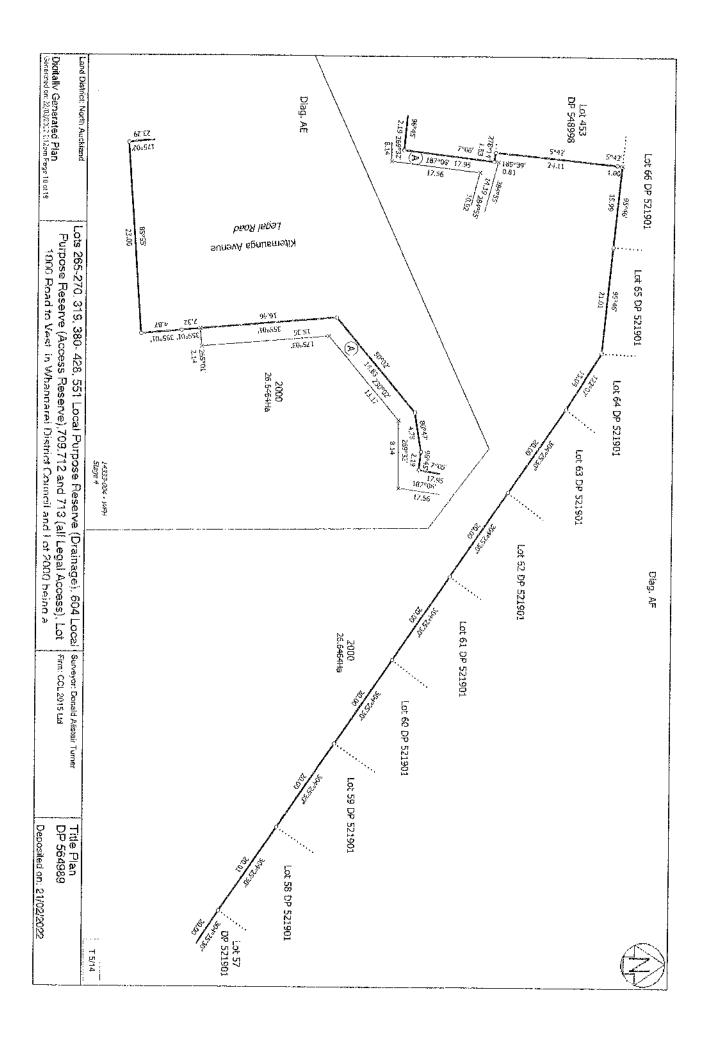
Lot 1000 is Road to Vest on Deposit with Whangarei District Council. Lot 551 is to Vest on Deposit for Local Purpose Reserve (Drainage) with Whangarei District Council. Lot 604 is to Vest on Deposit for Local Purpose Reserve (Access Reserve) with Whangarei District Council

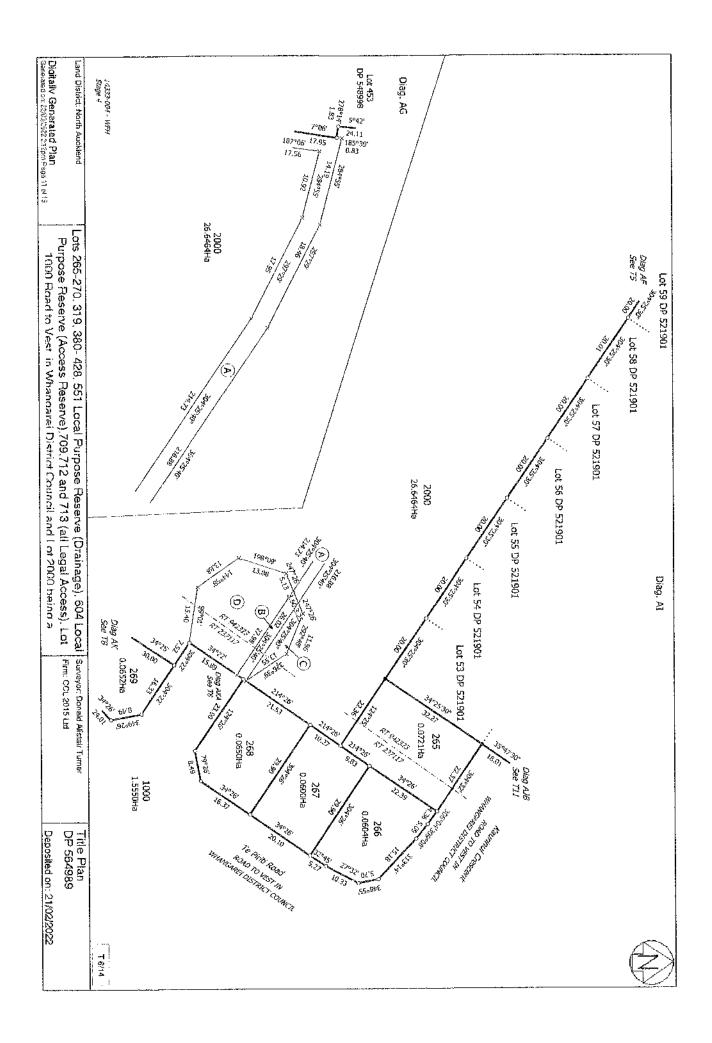


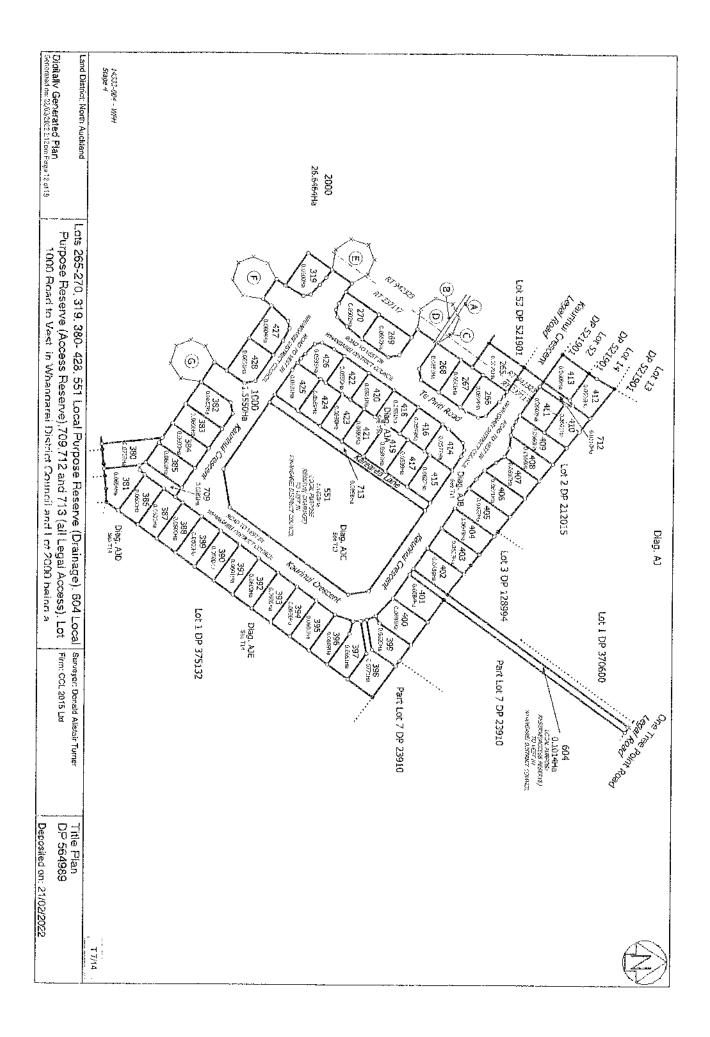


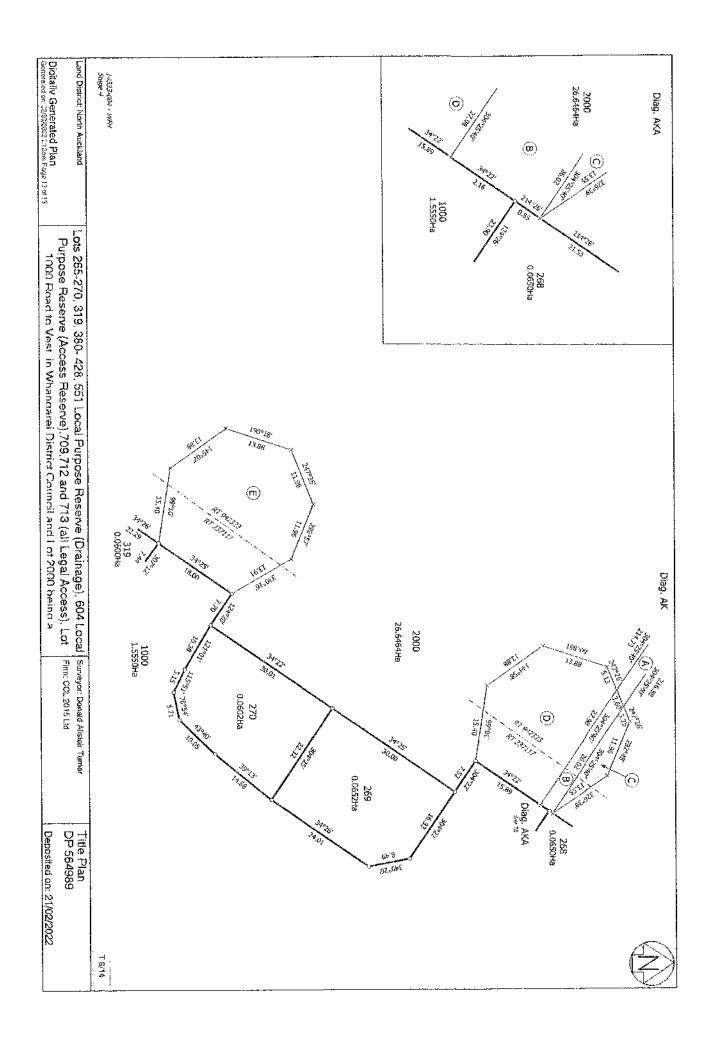


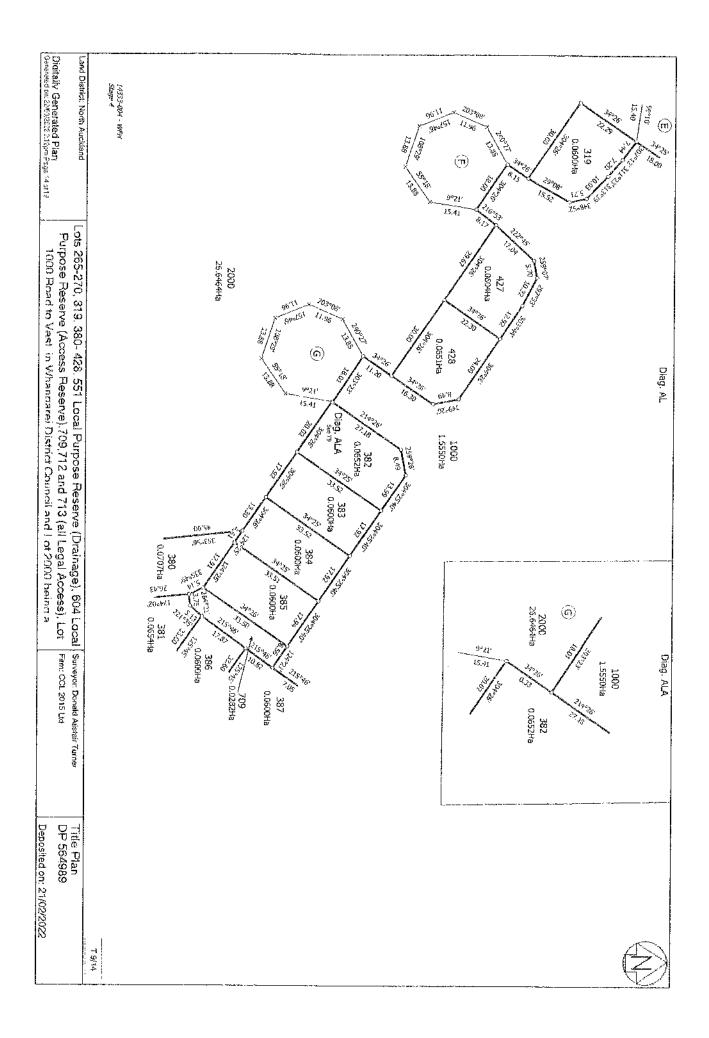


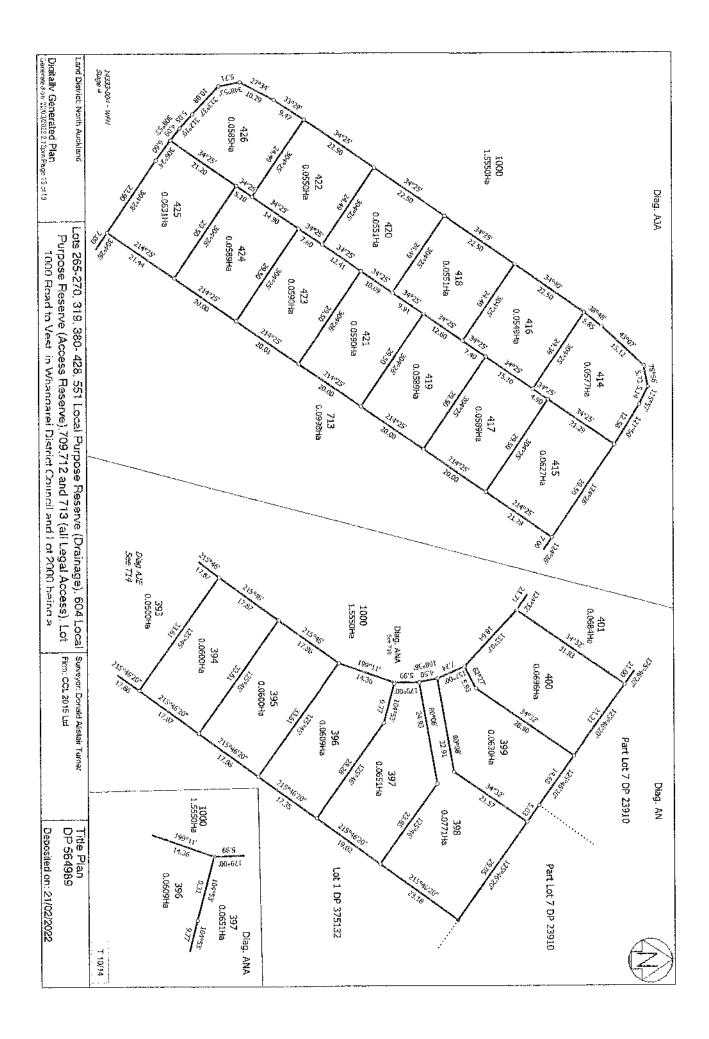


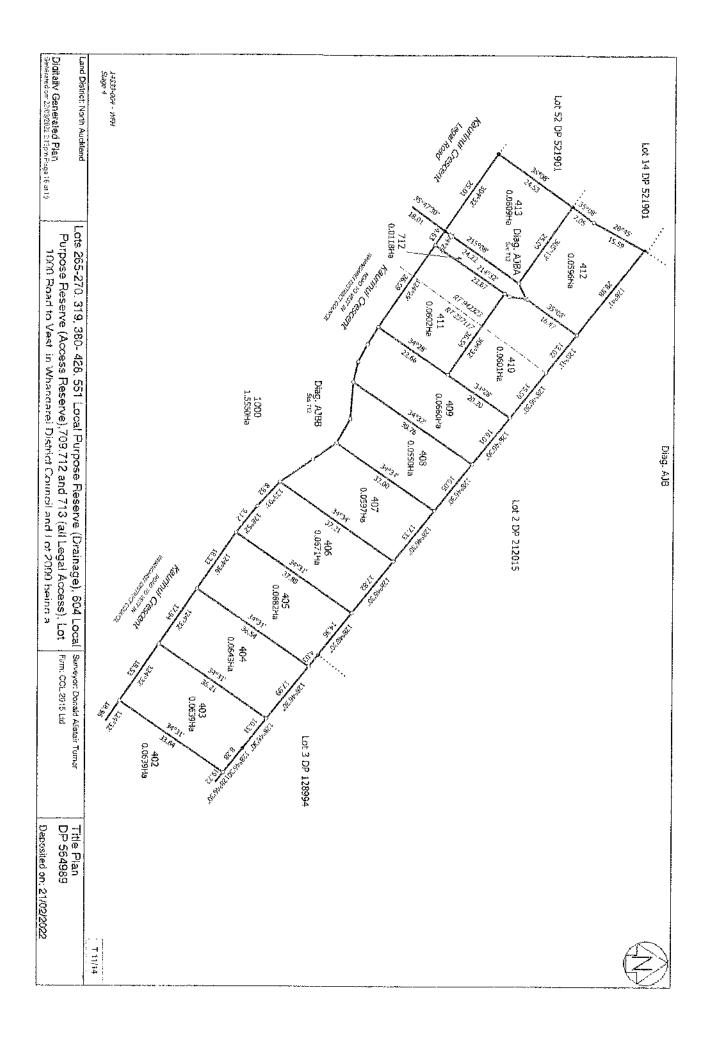


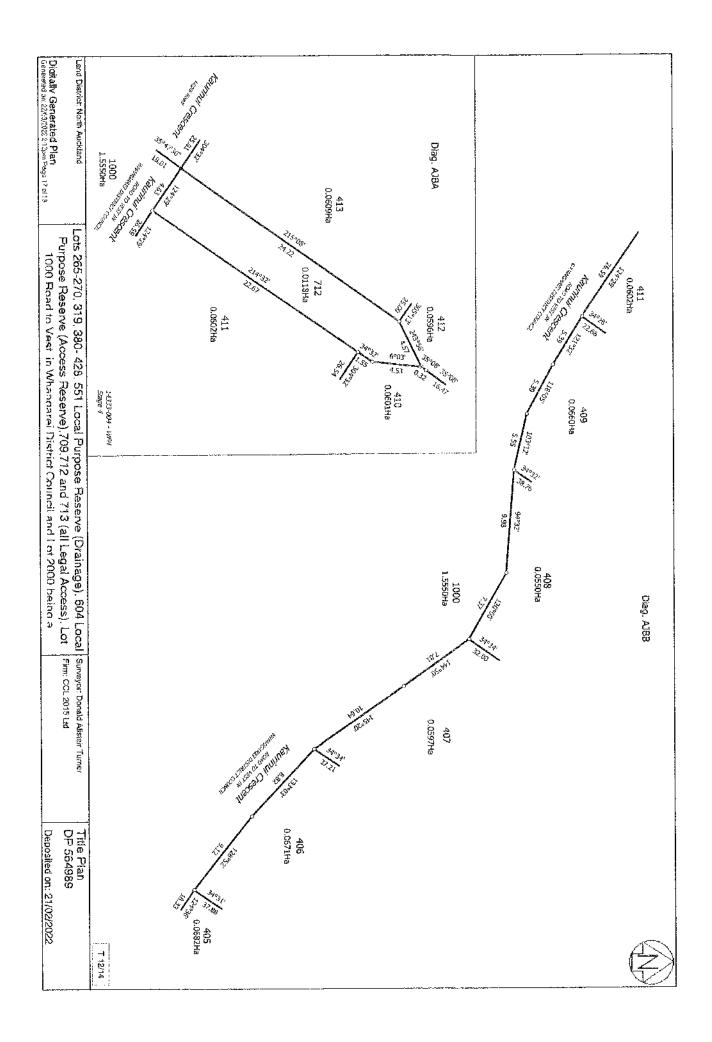


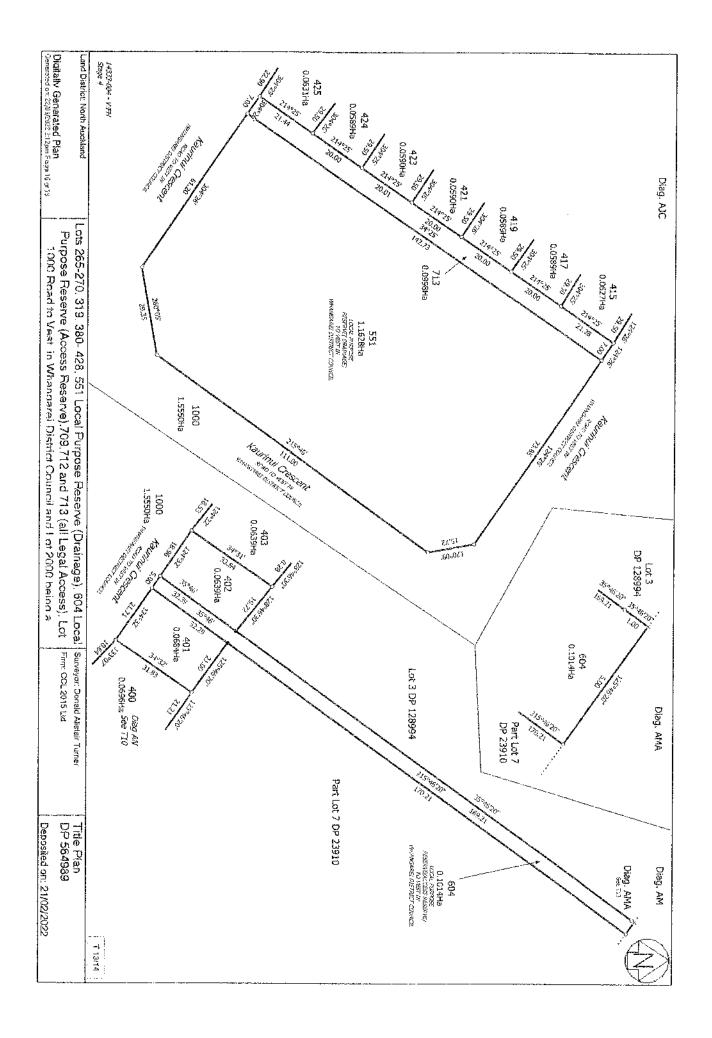


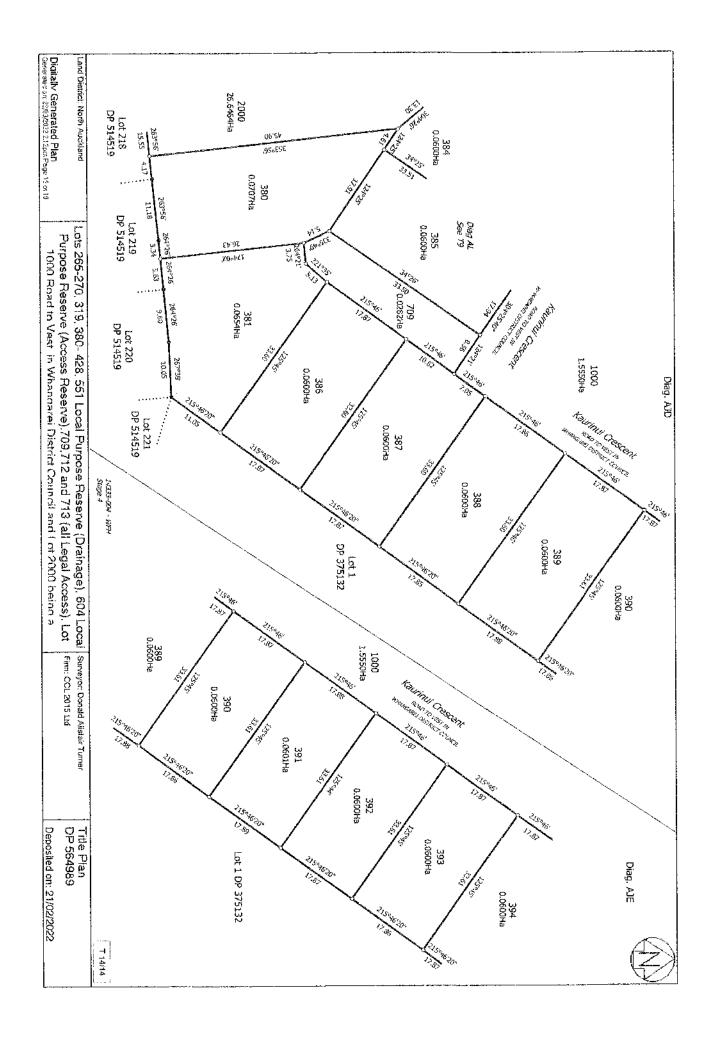














RECORD OF TITLE UNDER LAND TRANSFER ACT 2017 FREEHOLD

Search Copy



R.W. Muir Registrar-General of Land

ldentifier	1009463
Land Registration District	North Auckland
Date Issued	21 February 2022

Prior References 237117

Estate	Fee Simple	
Area	597 square metres more or less	
Legal Description	Lot 407 Deposited Plan 564989	
Registered Owners		
ATK Developments Limited		

Interests

12201143.6 Consent Notice pursuant to Section 221 Resource Management Act 1991 - 21.2.2022 at 4:16 pm

Land Covenant in Covenant Instrument 12201143.7 - 21.2.2022 at 4:16 pm

Land Covenant (in gross) in favour of WFH Properties Limited created by Covenant Instrument 12201143.7 - 21,2,2022 at 4:16 pm

Land Covenant in Covenant Instrument 12201143.8 - 21,2,2022 at 4:16 pm

IN THE MATTER	of the Resource Management Act 1991 ("the Act")
ARD	
in the matter	of a subdivision consent as evidenced by Land Transfer Plan No. 564959
AND	
IN THE MATTER	of a Consent Notice issued pursuant to Section 221 of the Act by WHANGAREI DISTRICT COUNCIL ("The Council")

and a second second second second second

IT IS NEREBY CERTIFIED that the following conditions to be complied with on a continuing basis by the subdividing owner and subsequent owners was imposed by the Council as a condition of approval for the subdivision as effected by Land Transfer Plan No. 564989 ("the plan")

In relation to lots 265-270, 319 and 380-428.

- At the time of building consent provide suitable evidence/design to illustrate that stormwater attenuation will be provided for all impervious surfaces exceeding 45% of the lot size area, to ensure compliance with Council's Environmental Engineering Standards 2010 and to the satisfaction of the Whangarel District Council. The design shall be undertaken by a suitably qualified engineer or Council IOP.
- 2. Any development shall comply with the restrictions and recommendations identified in the Earthworks Completion Report by Cook Costello – The Landing Stages 4 and 5 – Project Number 14333-004 dated 21 October 2021, available from Council under file reference SD1800110, unless an alternative engineering report prepared by a Geotechnical or Chartered Professional Engineer is approved in writing by Council.
- 3. At the time of building consent, the property owner shall arrange to supply and install (at its own cost) a pressure sewer system of an approved make, brand or type (including all its constituent components) to the satisfaction of Council. After approval by WDC Waste and Drainage Manager, and connection of the pressure sewer system to the Council's reticulated system, WDC shall own and maintain the pressure sewer system in accordance with the WDC Pressure Sewer Policy. The Owner shall continue to own and maintain the property's gravity drain to the pump chamber. The Owner shall be responsible for paying for the power consumption of the pumping unit in accordance with Council Pressure Sewer Policy.
- At the time of building consent, the owner shall construct the vehicle entrance crossing in accordance with Sheet 18 Residential Single Width Crossing also in ____

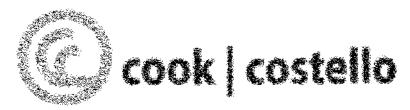
accordance with Sheets 22 & 23 of Council's Environmental Engineering Standards 2010 Edition unless an existing approved vehicle crossing to the site exist. Entrance crossings are to be designed and constructed in such a manner that will control stormwater run-off entering a property from the road, and that likewise prevent stormwater and detritus, including gravel, dirt and other materials, migrating onto the road reserve from a property.

DATED at Whangarei this 3rd day of November 2021.

SIGNED for WHANGAREI DISTRICT COUNCIL, pursuant to the authority of the Council given pursuant to the Local Government Act 2002 and the Resource Management Act 1991

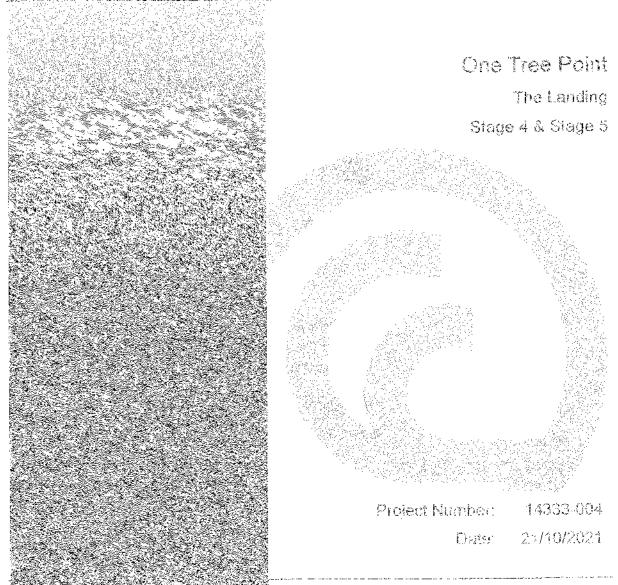
versale. Authorised Signalary -Karen Toni Šathedov Post Approval RMA Officer

NNF-126459-18-22-V1



The Landing Stage 4 & Stage 5 Earthworks Completion Report

WER PROPERTIES LIMITED



Whangarei I Auckland I Wellington I Chostehurch



The Landing - Stage 4 & 5

COOK COSTELLO DOCUMENT CONTROL RECORD

Client:	WFH PROPERTIES LIMITED
Project description:	The Landing Stage 4 & 5 - Earthworks Completion Report
Document name:	14333 - Stage 4 & Stage 5 - Earthworks Completion report_saved.docx
Address:	One Tree Point, Stage 4 & 5
Date of issue:	Thursday, 21 October 2021
Status:	For Issue

Stf-1ath

Originator:

Stefano Rotatori ~ Chartered Professional Engineer (Geotechnical) ME, IntPE (NZ), CMEngNZ, CPEng



Approved for issue:

Philip Cook - Chartered Professional Engineer

BE(hons),Dip Ag, CPEng, CMEngNZ, IntPE(NZ), MACENZ,MInstD

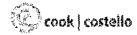


Whangarei

Office of origin:

Version Date Comment By 1.0 14 September 2021 Review P. Cook 2.0 17 September 2021 Issue S. Rotatori 3,0 21 October 2021 Issue S. Rotatori

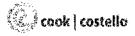
The Landing Stage 4 & Stage 5 - Earthworks Completion Report



WFH Properties Limited	3
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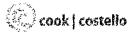


WFH Properties Limited

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1. Limitations

This report, and the stated or inferred professional opinions found within, relating to the suitability of the areas being ready for building development, shall not be construed as a guarantee.

This report, and the inferred professional opinions found within does not remove the necessity for sitespecific geotechnical investigation, normal inspection and design of foundations for each lot, as would be made in natural ground and NZS3604 for standard residential building foundations.

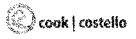
2. Executive Summary

Cook Costello was engaged by WFH Properties Ltd, to provide technical advice for earthworks and be the geo-professional for the Landing Stage 4 & 5 subdivision at One Tree Point, Northland. Cook Costello designed earthwork cuts and placement of controlled sand engineered fill, in order to enable construction of the building platforms within the proposed development.

A requirement of the consenting authority WDC (Whangarei District Council) is that a "Statement of Professional Opinion on Suitability of Land for Building Construction" (Statement of Suitability) is made by the geo-professional at the end of construction (prior to building developments). This process requires "WDC form EES-PO1" to be completed, and requires a geotechnical completion report. The completion report summarises the situation and is used to describe any departure from the standard foundation conditions expected by the building industry and outline any restrictions or recommendations if required. The purpose of this report is to provide the supporting details required for the Statement of Suitability.

The Statement of Professional Opinion on Suitability of Land for Building Construction - Form EES-PO1 has been attached to Appendix 6.

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3. Introduction

Cook Costello was engaged by WFH Properties Ltd, to provide technical advice for earthworks and be the geo-professional for the Landing Stage 4 & 5 subdivision at One Tree Point, Northland. Cook Costello designed earthwork cuts and placement of controlled sand engineered fill, in order to enable construction of the building platforms within the proposed development.

Previous reports and geotechnical information at the site comprise of:

- Ormiston Associates Ltd. Geotechnical Report (dated September 2005).
- Cook Costello Earthworks Specification (dated October 2018).

This report is for completion of the subdivision works in relation to the developed sections and covers the information required by the Whangarei District Council for preparing consent notices for the future property titles and provides geotechnical information to the future lot owners. It covers the requirements of the Whangarei District Council Engineering Standards (Issue 1, operative from 1 July 2010) for subdivision completion and is an addendum to the Form EES-PO1 certificate.

On completion of the works, the developer shall provide the following:

- Geotechnical reports and plans,
- All limitations on the development of the properties, including hazards, easement requirements etc.

This report relates to the lots in Stages 4 & 5 of the subdivision. The lot numbers for Stage 4 & 5 (as shown on the Scheme Plan appended to Appendix 1) covered by this report are:

Stage 4:

- 56 Residential Lots (lots 265 270; 319; 452; 380 428).
- Roads 4, 5, and 8.

Stage 5:

- 45 Residential Lots (lots 250 264; 447 452; 271 291; 316-318).
- Roads 6, 7, 10 and 19.

The subdivision has been designed and constructed to satisfy Whangarei District Council Environmental and Engineering Standards 2010.

4. Sile Description

The Landing subdivision is situated at One Tree Point, approximately 17 km southeast of Whangarei Central Business District. The subdivision consists of 9 stages, with Stage 1, Stage 2, Stage 3, Stage 4 and Stage 5 completed and Stage 6 and Stage 7 currently under construction. The scheme plan for Stages 4 & 5 can be seen in Appendix 1.

The property is regionally placed as part of the Marsden Point Barrier Spit, a broad peninsula, comprised of coastal SAND dunes, estuarine and alluvial sediments, that partly encloses Whangarei Harbour. These are considered young soils.

The spit has been built up by a prograding (seaward advancing) foreshore depositional process in the vicinity of the harbour entrance with sequences of aeolian influenced coastal SAND dunes and alluvial and estuarine sediments.

The GNS Science online geology map defines the underlying geology of the site as comprised of Late Pleistocene stable dune deposits. Weakly cemented sand in fixed transverse dune ridges.

The soil type in the area is defined on NZMS290 Sheet Q06/07 Hukerenui - Whangarei (SOILS) as a combination of Ruakaka Loamy Peat, One Tree Point Peaty Sand and Tangitiki Sandy Loam and Sand (imperfectly to poorly drained).

The rock type in the area is defined on NZMS290 Q06/07 Hukerenui - Whangarei (ROCK TYPES) as Sand: felspathic with some quartz, minor dark minerals and clay forming fixed dunes, minor swamp deposits; unconsolidated to very soft. Unweathered or weathered to brown-stained very soft clayey sand to depths of 5m.

6. Construction Methodology

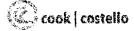
The earthworks were carried out under Cook Costello as the supervising engineering consultants with Geocivil Ltd carrying out the physical testing. The general methodology consisted of a removal of organic material (peat and topsoil) stripped ground inspection, cut to fill, with fill being tested, and covering complete fill with organic topsoil peat.

A bulk earthworks plan showing the finished earthworks and contours is included in Appendix 2.

8.1. Earthworks

A bulk earthworks operation was carried out over the site to improve the drainage and contour of the Lots. Earthworks were also carried out to establish reasonable building platforms. Stage 4 & 5 of the Landing used the following methodology:

- The topsoil and the peat were stripped off using motor scrapers and 50 ton Moxy dump trucks (Cat 725) with 20 – 30 ton excavators (Cat 320).
- 2. Cut to fill in the old sand dune complex using Sand fill was undertaken to bring the lots up to finished level.
- Testing was undertaken to achieve a medium dense compacted sand with an allowable bearing capacity of greater than 100 kPa (Ultimate Bearing Capacity of 300kPa). This meets the requirements of the New Zealand Standard 4404 and the New Zealand Building Code.



WFH Properties Limited

The Landing - Stage 4 & 5

- 4. Once the desired contours were achieved topsoil was spread from the temporary stockpile using Motor scrapers. This was track rolled with a D6 dozer and levelling bar.
- 5. Grassing of Stage 4 & 5 occurred very shortly after the top soiling process was completed.

\$.2. Roading

A standard industrial/commercial roading operation was undertaken on Stage 4 & 5. The roading has been achieved using the following methodology:

- 1. Trim subgrade sand layer to level using Motor Scrapers and graders.
- 2. Check the subgrade CBR by Scala Penetrometer analysis,
- 3. Place lay and spread AP65 subbase layer using a Grader and 10 ton dynamic and static rollers to achieve optimum density.
- 4. Prepared Kerb lines using a grader and a 4 ton Dynapac roller.
- 5. Place, Laid, and formed slip form kerb and channels.
- 6. Backfilled kerb & channel using 12 Ton excavator.
- 7. Place lay and spread AP40 basecourse layer using a grader and 10 ton dynamic and static rollers to achieve optimum density, using a 9 ton PTR to tighten the top layer ready for sealing.
- 8. Check the compaction of the basecourse layer by Nuclear Densometer Readings and Benkelman Beam testing.
- 9. Sweep / clean the road of loose material and debris ready for seal.
- 10. Seal the street with waterproofing seal coat and single coat chip seal (grade 5) using automatic pressurised bitumen sprayer 6-wheel trucks with chip spreading boxes on to spread both the bitumen and chip over the surface evenly.
- 11. Apply asphaltic concrete over waterproofing layer using paving machine and rollers. Asphaltic concrete to be DG10 or AC10 (refer to paving plan) to comply with NZTA M/10 Specification.

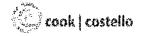
7. Testing & inspection Methodology

The relevant quality control testing for the bulk earthworks was undertaken by Cook Costello and Geocivil Ltd at the required intervals. Testing locations are appended in Appendix 3, results of the tests can be found in Appendix 4.

Supervision by the Engineers was required at key critical stages, and a series of hold points were identified and agreed with the Contractor.

7.1. Fill Testing

The Cook Costello Earthworks Specification (dated October 2018) initially suggested testing consisting of Light Weight Deflectometer (LWD), Scala Penetrometers (DCP) and Nuclear Densometers (NDM).



However, during compaction of the material, it was discovered that there was a delay of approximately 4 – 5 weeks between compaction and achieving the desired compaction results. This is due to porepressure build up in the compacted fill material caused by the compaction process. The delay between testing and receiving results was unacceptable in terms of constructability.

As a result, it was decided to test the fill material using Static Plate Load Tests (PLT) where fill to undercut levels was deeper than 1.5m. This allowed the testing to be conducted at the finished level, speeding up the construction process. DCPs, NDMs and LWDs were conducted at the finished level after the excess pore pressure was allowed to dissipate where fill depth to undercut was less than 1.5m depth.

Frequency of testing has been set up to be not less than:

- 1 test per layer or 200 mm thickness per material type per 2500 m²; or
- 1 test per 500 m³ distributed reasonably evenly throughout full depth and area; or
- 3 tests per visit; whichever requires the most tests.

As a result, the following testing has been carried out on the sand FILL areas:

- No. 103 Plate Load Tests
- No. 93 Scalas/NDMs/LWDs
- No. 5 CPTs

Locations of testing on the fill areas are shown on Drawing G01, appended to Appendix 3.

To be noted that the 300kPa ultimate bearing capacity has been calculated according to DIN 18134:2012-04 at 5mm vertical settlement (DIN 18134:2012-04: German Standard Soil Testing procedures and testing equipment -- Plate load test, English translation of DIN 18134:2012-04).

7.2. Undercut Testing

The Cook Costello Earthworks Specification (dated October 2018) suggested testing consisting of Light Weight Deflectometer (LWD), Scala Penetrometers (DCP) and Nuclear Densometers (NDM). Prior to fill placement, the area was stripped of vegetation and topsoil and inspected by Cook Costello and Geocivil Ltd. Site testing with DCPs was conducted to identify areas and depths that required further undercutting. Undercutting terminates on the consolidated sand layers (i.e. hard pan) and/or at the bottom of the peat or sandy peat layer.

Enabling works, undercut and benching to competent material were inspected and approved by the supervising Engineer prior to filling.

Approximately 100 Scala penetrometer tests were carried out to assess the suitability of the undercut areas.

Locations of testing on the undercut areas are shown in Drawing G02, appended to Appendix 3.

7.5. Laboratory Testing

Laboratory testing was undertaken during construction to determine the following characteristics for the sand fill and for enabling site testing procedures. Laboratory testing is attached to Appendix 3. An Exhaustive list of laboratory testing is reported below:

- Determination of the Dry density/water content relationship -- New Zealand Standard Compaction - NZS 4402: 1986 Test 4.1.1
- Determination of the particle size distribution-Dry sieving method NZS 4407:2015 Test 3.8.2
- Determination of the Dry density/water content relationship ~ New Zealand Heavy Compaction
 NZS 4402: 1986 Test 4.1.2
- Determination of Organic Content.
- Determination of PH(f) and PH(ox) in soil.

8. Poundation Design

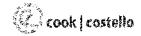
8.1. Officiate Searing Capacity

Investigations have been carried out by or under the supervision of Geocivil Ltd or Cook Costello. The locations of tests are shown in Appendix 3. Test results are shown in Appendix 4.

In accordance with the Whangarei District Council Environmental Engineering Standards dated 2010 clause 2.7 The Councils Report on Completion of Construction undertook testing as the excavation or filling was carried out. The following testing was undertaken to support the statement of professional opinion as to the compliance of the filled/cut ground to the specification and the suitability of the land for Building Construction. (WDC Form EES-PO1). The testing undertaken in our view provides sufficient information to allow identification of any specific design requirements that necessitate the building foundation design to deviate from NZS 3604 and NZS 4229 and design parameters for detailed design of the foundations (such as bearing capacity, suitable founding depth etc).

The testing approach has been to test and demonstrate that the bearing capacity requirements of NZS3604 and NZS 4229 have been met or exceed the requirements of the definition of "Good Ground" where the tests were undertaken. It is assumed that the extent of testing represents that 300kPa ultimate bearing capacity is exceeded over the entire building platform even though not every piece of that ground/soil has been tested.

The existing depth of peat/peaty sand was determined by the Geotechnical Investigation Report carried out by Ormiston Associated Ltd for the residential subdivision (ref. # 1600/2114, dated September 2005). 51 CPTs, 25 Flight Augers, and 4 test pits were carried out on site. These tests showed a depth generally up to 5.5m of inferred organic peat, peaty sand and organic silts with good sand bearing below, over the majority of the site. Due to the ongoing long term settlement and shrinkage properties of the peat and organic solls, it was decided to remove this material as part of the bulk earthworks operations.



Scala Penetrometer and Plate Load testing were carried out to identify the compaction and ultimate bearing capacity of the sand fill layers. Scala Penetrometers were used to determine ultimate bearing capacity in cut areas. The bearing capacity results are summarised in Table 1. The provided ultimate bearing capacity of 300kPa is available below any topsoil that has been spread on site after the completion of the earthworks. To be noted that between 100 - 300 mm of topsoil has been spread throughout the site with an average of 200 mm.

Table i: Rearing capacity suprastly for Stage 4 & Stage 5 of The Landing Subdivision.

Anga	Dei	oli (mbgl)	Vinneté Static Bearing. Capachy (kPa)	Dependable Static Rearing Capacity (kRa)
Stage 4 & 5 (All Lots)	0.1	- 0.3	300	150

The Statement of Professional Opinion on Suitability of Land for Building Construction - Form EES-PO1 is included within Appendix 6.

8.2. Static Settlements

Foundation design should limit the probable maximum differential settlement over a horizontal distance of 6 m to no more than 25 mm under serviceability limit state load combinations of AS/NZS 1170 Part 0, unless the structure is specifically designed to prevent damage under a greater settlement.

8.3. Subson Class

Generally, across the entire site, the soils are consistent with site subsoil classification Class C - Shallow Soil sites as per NZS1170.5, based on estimated shear velocity (Vs) values from the CPTs presented within Ormiston Associated Ltd (ref. # 1600/2114, dated September 2005) and the calculated natural period of the site was less than 0.6 seconds.

8.4. Liquetaction Potential

8.4.1. Previous Liquefaction Analysis

As stated in the Geotechnical Investigation Report carried out by Ormiston Associated Ltd (ref. # 1600/2114, dated September 2005). The site presents a low risk for the development of liquefaction during an earthquake. The investigations at the property presented in the Ormiston Geotechnical report, in particular the CPT tests, indicates that the site is underlain by upper dense sands (qc varying from 8MPa to 20MPa) overlying a layer of loose to medium dense sands.

Groundwater is variable across the site, varying between 3.04m and 5.9m. Liquefaction does not occur above the groundwater table and this relatively deep groundwater decreases the potential of liquefaction within the shallow layers that have been cut up to the consolidated sands (hardpan) or filled using compacted sands.



In the sand-filled areas, where the unsuitable materials have been removed, sand fill has been compacted, increasing the relative density of the sands. Subsequently, further CPTs testing have been carried out to assess the liquefaction potential of the compacted sand fill. Liquefaction analysis is discussed in the next Section 1.1.1.

6.4.2. OPT investigations

Geocivil Ltd conducted three CPTs to a 20 m target depth on 10 June 2020 where the sand fill depths were more significant within Stage 4 and Stage 5 subdivisions. To be noted that between 100 – 300 mm of topsoil has been spread throughout the site. In fact, the top 300 mm layer encountered within the CPTs records show a cone resistance between 2 and 6 MPa as recorded within the topsoil layer. However, all the test results show Dense / Very Dense Sand below the topsoil with a cone resistance gc between 10 and 20 MPa.

CPT investigations carried out on site are summarised in Table 2. Detailed testing results are shown in Appendix 4, CPTs test locations are shown in Appendix 3.

Ţest iB	Depth (ni)	Groundwater Table (m)	Liepth (m)	qc (MPa) ⁺	laterpleted Sail: Description													
CPTF	4.50	Not	0 - 0.25	3	Silty SAND and Sandy SILT													
	1.36	Encountered	0.25 – 1,36	18	SAND and Silty SAND													
CPTG	0,86	Not	0 - 0.45	5	SAND and Silty SAND													
616	0.00	Encountered	0.45 - 0.86	20	SAND													
	3.46		0.0 0.35	4.5	Silty SAND and Sandy SILT													
CPT I			0.35 - 0.80	12	SAND and Silty SAND													
		3.46	346	Not Encountered												0.80 - 1.70	16	SAND
			1.70 - 3.20	13	SAND and Silty SAND													
			3.18 - 3.46	20	SAND													
	15.6	5.2	0.0 2.6	9.5 - 12	SAND and Silty SAND													
CPTL			2.6 - 3.0	5	Silty SAND and Sandy SILT													
			0.2	3.0 - 9.5	6 - 20	SAND and Silty SAND												
			9.5 -	4-9	Silty SAND and Sandy SILT													

1. Cone tip resistance

8.4.3. Yeak Ground Acceleration

According to NZS 1170.5:2004, Importance Level 2 buildings are required to be designed to resist earthquake shaking with an annual probability of exceedance of 1/500 (i.e. a 500-year return period).

WEH Properties Limited The Landing - Stage 4 & 5

This is the ultimate limit state (ULS) design seismic loading. Structures are expected to retain their structural integrity during the ULS earthquake, and not collapse or endanger life.

Peak horizontal ground accelerations (PGA) have been calculated in accordance with MBIE/NZGS Module 1 (2016) using the following formula:

 $PGA = C_{0,1000} R f g / 1.3$

Co,1000 = 0.13 for Whangarei (NZTA Bridge Manual Commentary (2018) Table C6)

R = 1.0 for a 500-year return period event (NZS 1170.5)

f = 1.33 for Class C

Thus, the PGA = $0.13 \times 1.0 \times 1.33 \text{ g} / 1.3 = 0.13 \text{ g}$ for ULS.

The seismic parameters are summarised in Table 3.

Table	З;	Seismic	parameters.
-------	----	---------	-------------

Importanțe Level	Limit State	Peak Ground Acceleration, PGA or (11)	Effective Magnitude, My
2	ULS ¹	0.13	5.8
2	SLS ²	0.03	5.8

1. Ultimate Limit State

2. Serviceability Limit State

S.4.4. Method of Analysis

CPT results have been analysed using CLiq software to assess the potential soil liquefaction and settlement. Boulanger & Idriss (2014) was used to evaluate the liquefaction potential (triggering) of the soil strata at the site, the method proposed by Zhang et al. (2002) was used for settlement calculations.

Liquefaction analysis results are attached in Appendix 5.

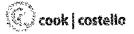
The assessment of liquefaction has been undertaken under ultimate limit state design (ULS) criteria and serviceability limit state design (SLS) criteria. The serviceability limit state is the point where a structure can no longer be used for its intended purpose but would still be structurally sound. The tolerances for serviceability depend on the intended use of the structure and can vary significantly (Section 2 of NZS 1170.5:2004).

A groundwater level of 3.0 mbgl was conservatively considered during earthquake events for the liquefaction analysis which is the shallowest recorded value within the Geotechnical Investigation Report carried out by Ormiston Associated Ltd.

8.4.5. Liquefaction Severity Number (LSN)

The Liquefaction Severity Number (LSN) is a method developed by Van Ballegooy et al (2013) which provides an estimate of liquefaction damage manifesting at the ground surface. The LSN parameter

The Landing Stage 4 & Stage 5 - Earthworks Completion Report



WFH Properties Limited	14
The Landing – Stage 4 & 5	

refines the calculated settlement by including a depth weighting function and can be used to differentiate the most severe damage land from the least affected land. Table 4 provides the LSN ranges and associated damage classifications.

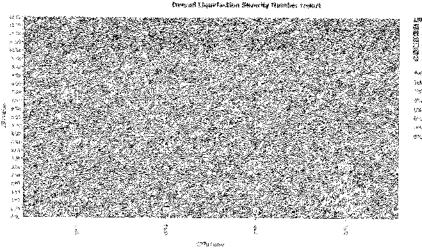
Len Range**	Birninge Clessification
0 – 10	Little to No expression of liquefaction
10 - 20	Minor expression of liquefaction
20 – 30	Moderate expression of liquefaction
30 - 40	Moderate to severe expression of liquefaction
40 - 50	Major expression of liquefaction
50+	Severe damage

8.4.6. Altimate Limit State

A PGA value of 0.13 g and an earthquake magnitude (Mw) of 5.8 have been used for liquefaction analysis in accordance with NZGS Module 1 and the NZTA Bridge Manual guidelines.

8.4.8.1. Liquefaction Severity Number

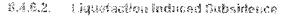
The LSN for the ULS design case was 2.6 only for CPTL and nil for CPTF, G and I. This indicates Little to no expression of liquefaction according to Table 4. Refer to Figure 1 below.



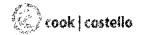


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Figure 1: LSN results summary (ULS).



up to 20 mm of settlement has been noted under the ULS design earthquake parameters for CPTL. Refer to the liquefaction results attached in Appendix 5.



The Landing - Stage 4 & 5

Lateral Oisplacement 8.4.6.3.

Lateral displacement is not considered to be an issue on site due to the presence of relatively flat ground.

a.4.7. Serviceability Limit State

A PGA value of 0.03 g and an earthquake magnitude (Mw) of 5.8 have been used for liquefaction analysis for the SLS case in accordance with NZGS Module 1 and the NZTA Bridge Manual guidelines.

Liquefaction Severity Number 8.4.7.1.

The LSN for the SLS design case was nil. This indicates Little to no expression of liquefaction according to Table 4. Refer to Figure 2 below.

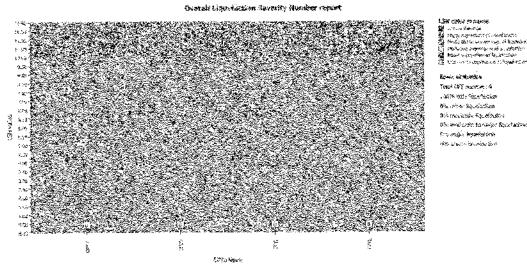


Figure 2: LSN results standary (SLS).

Liggefaction Induced Subsidence 8.4.7.2.

No settlement has been noted under the SLS design earthquake parameter. Refer to the liquefaction results attached in Appendix 5.

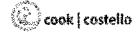
8.4.8. Liquefaction Summary

Liquefaction potential is considered to be low. Little to no expression of liquefaction is expected within Stages 4 & 5 subdivision. Liquefaction analysis results are attached in Appendix 5. Up to 20 mm of settlements could be expected under the ULS earthquake case that are within the tolerance of the building code that is 25 mm over 6 m according to Clause B1/VM1 for static settlements.

8.5. Acid Subphate Risk

8.5.1. Acid Sulphate Testing Discussion

The Landing Stage 4 & 5 subdivision is mapped on the Whangarei District Council Map to be in the Acid Sulphate Risk Area. Acid Sulphate Soil Field PH Tests have been conducted on site following the Acid



an brands for

Sulfate Soils Laboratory Methods Guidelines, Version 2.1, Section H, published by Department of Natural Resources, Mines and Energy, Indooroopilly, Queensland, Australia, June 2004. This Australian guide applies to soils below an RL of 5.0m.

Given the vicinity with Stage 3, no further testing was carried out for Stages 4 & 5. Outcomes of further testing would not have changed the conservative approach the mitigation measures for foundation design recommended in Sections 8.5.2 and 8.5.3.

Field pH (pHF) recording has been undertaken on site at the invert level (Test B) of proposed infrastructures (i.e. stormwater and sewer pipes) and 1 m above the invert level (Test A) in 10 different locations. Field testing is appended to Appendix 4.

For both Test A and Test B cases, field pH peroxide test (pHox) was performed on each of the 10 samples adding 30% of H2O2 (Hydrogen peroxide) to the 10 samples taken from the field in order to determine soil pH following complete oxidation.

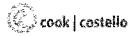
After the reaction, the rate of the reaction indicates the level of sulfides present. The reaction is characterised by heat and gas evolution. A soil containing very little sulfides only rate an 'X' (Slight Reaction) however a soil containing high levels of sulfides is likely to rate an 'XXXX' (Very Vigorous Reaction). All the soil reaction of the pHox tests undertaken by Geocivil are characterised by an "X", therefore soils are likely to contain very little sulfides. Also, according to the ASS Sampling & Analysis Guidelines, 1998, a pHox value at least one unit below field pHF indicates potential presence of acid sulphate soils. The greater the difference between the two measurements, the more indicative the value is of a PASS (Potential Acid Sulphate Soils). The lower the final pHox value is, the better the indication of a positive result. The lab testing generally shows a pHox value less than one unit below field pH, therefore the presence of acid sulphate soils could be little. Another qualitative indication of little sulphides is that If the measured pHox < 3. The more the pHox drops below 3, the more positive the presence of sulfides is. There is one pHox value recorded below 3.

These results are consistent with local observations of good quality concrete that had been in the ground for a long time and has been excavated and shows no sulphate acid damage.

However, the PHF and pHox values are comprised between 2.92 and 5.22. According to NZS 3101:Part 1:2006 Section 3.4.3.2, an acidity represented by a PH of 5.0 to 5.5 may be considered as a practical limit of tolerance of high-quality concrete in contact with any acids. For pH lower than 5.0, the environment shall be assessed as exposure classification U. As per Section 3.8, NZS 3101, Exposure Classification U represents an exposure environment not specified in Table 3.1 of NZS 3101 for which the degree of severity should be assessed by the designer. Concrete in members subject to exposure classification U shall be specified to ensure durability under the particular exposure environment and for the chosen design life. Protective coatings may be taken into account in the assessment of concrete requirements.

Alternatively, as reported in the Acid Sulphate Soll Planning Policy Basic Guide prepared for WDC by Opus International Consultants, Australian Standard AS 2159 – 2009 Section 6 is relevant when

The Landing Stage 4 & Stage 5 - Earthworks Completion Report

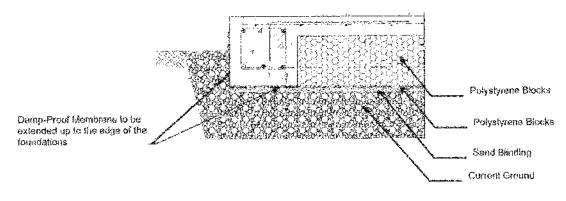


WFH Properties Limited The Landing – Stage 4 & 5

designing concrete and steel structures to withstand corrosive environments. This standard refers to piles, however the knowledge can be transferred to other structures.

8.5.2. Mitigation measure for Shallow Foundations

Given the acid sulphate soils risk and low pH values recorded on site, it is recommended installing a waterproof membrane underneath any shallow foundations, the membrane should be extended up to the edge of the foundations. If the above recommendations are followed, concrete exposure class can be A1 according to NZS3101. A typical detail is shown in Figure 3 showing the damp-proof membrane for a typical foundation detail.



Siguro 3: Darep Proof Membrane Defail

\$ 5.3. Mitigation measure for Deep Foundations

8.5.3.1. Concrete

In case of deep concrete foundations were used on site, it is recommended referring to the Australian Standard AS 2159 – 2009 Section 6 when designing concrete and steel structures to withstand corrosive environments.

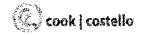
The exposure classification of concrete is *Very Severe* according to Table 6.4.2 (C) of AS 2159 – 2009, considering a pH less than 4 and soil Conditions A (high permeability soils). It is recommended to refer to Table 6.4.3 of AS 2159 – 2009 to select the minimum strength of concrete and concrete cover relevant for the *Very Severe* Exposure Classification and design life.

5.5.3.2. Steel

If steel elements are used in the ground (i.e. driven H-steel piles, etc.), it is recommended adopting the Severe Exposure Classification shown inTable 6.5.2 (C) of AS 2159 – 2009, considering a pH less than 3 and soil Conditions A (high permeability soils).

A uniform corrosion allowance of 0.04-0.1 mm/year should be adopted during the design of any underground steel structure.

The Landing Stage 4 & Stage 5 - Earthworks Completion Report



WFH Properties Limited

The Landing - Stage 4 & 5

8.8.3.3. Timber

No precautions are required to be taken for timber piles. Timber is not susceptible to corrosion by acid.

8.6. Proposed Building Foundations

The test results indicate that the sites (once the peat / topsoil layer is removed) are suitable for standard foundations for buildings.

If a shallow concrete strip footing or concrete slab on grade is to be used, any organic soil such as peat and topsoil in the building footprint is to be excavated and replaced with an engineered hard fill layer of compacted aggregate in accordance with NZS 4431 or AS 3798:2007.

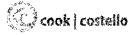
The Sandy material for all lots in Stage 4 & 5 is consistent with a Class A site as defined by AS 2870;2011 indicating little to no ground movement from moisture changes is expected.

This means that the finished ground within the lots is suitable for the erection of buildings on "Good Ground" in terms of NZS 3604:2011 (Timber Framed Buildings) & NZS 4229:2013 (Concrete Masonry Buildings Not Requiring Specific Engineering Design) and related documents providing that:

- Standard check of excavated foundations is carried out at time of construction.
- Buildings subject to heavy loads or vibrations will require specific design.
- Buildings not meeting the criteria for NZS3604:2011 and NZS4229:2013 will require specific design.

This report and the inferred professional opinions found within does not remove the necessity for normal inspection and design of foundations, as would be made in natural ground.

For the additional investigation, test sites should be situated under the building areas for each site and should be selected to give adequate information about the soil over the entire plan area of the proposed building. There shall be a minimum of four test sites for a building up to 200 m², with at least one additional test site for each additional 100 m² plan area of building as recommended by NZS3604.



The Landing - Stage 4 & 5

3. Conclusions

Cook Costello was engaged by WFH Properties Ltd, to provide technical advice for earthworks and be the geo-professional for the Landing Stage 4-5 subdivision at One Tree Point, Northland. Cook Costello designed earthwork cuts and placement of controlled sand engineered fill, in order to enable construction of the building platforms within the proposed development.

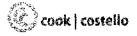
All lots have been subject to bulk earthworks to provide sites suitable for standard foundations for typical residential buildings. Any heavy structures or heavy industry users will require specific design and further investigations. A layer of topsoil was spread over the lots to ensure moisture is retained and grass will grow to prevent wind-blown erosion.

Based on our investigations we make the following conclusions and recommendations:

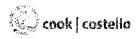
- The test results indicate that the sites (once the peat/topsoil layer is removed) are suitable for standard foundations for buildings.
- An ultimate bearing capacity of 300kPa is available below any peat / topsoil that has been spread on site after the completion of the earthworks. To be noted that between 100 300 mm of topsoil has been spread throughout the site with an average of 200 mm.
- The site is consistent with Subsoil Class C Shallow Soil sites as per NZS1170.5.
- Liquefaction potential is low. Little to no expression of liquefaction is expected within Stage 4 and Stage 5 subdivisions.
- Up to 20 mm of settlements could be expected under the ULS earthquake case that are within the tolerancy of the building code that is 25 mm over 6 m according to Clause B1/VM4 for static settlements.
- Foundation design should limit the probable maximum differential settlement over a horizontal distance of 6 m to no more than 25 mm under serviceability limit state load combinations of AS/NZS 1170 Part 0, unless the structure is specifically designed to prevent damage under a greater settlement.
- The Landing Stages 4 & 5 subdivisions are mapped on the Whangarei District Council Map to be in the Acid Sulphate Risk Area that is a well-known hazard in the area and well addressed with standard construction measures (i.e. waterproof membrane for shallow foundations).

Acid Sulphate Soil Field PH Tests have been conducted. Results of lab testing suggest that soils are likely to contain very little sulfides.

However, the PHF and pHox values are comprised between 2.92 and 5.22. According to NZS 3101:Part 1:2006 Section 3.4.3.2, an acidity represented by a PH of 5.0 to 5.5 may be considered as a practical limit of tolerance of high-quality concrete in contact with any acids.



- For pH lower than 5.0, the environment shall be assessed as exposure classification U as per NZS 3101.
- Alternatively, as reported in the Acid Sulphate Soil Planning Policy Basic Guide prepared for WDC by Opus International Consultants, Australian Standard AS 2159 – 2009 Section 6 is relevant when designing concrete and steel structures to withstand corrosive environments.
- It is recommended installing a waterproof membrane underneath any shallow foundations, the membrane should be extended up to the edge of the foundations to mitigate the presence of corrosive environment. If the above recommendations are followed, concrete exposure class can be A1 according to NZS3101.
- In case of deep concrete foundations were used on site, it is recommended referring to the Australian Standard AS 2159 – 2009 Section 6 when designing concrete and steel structures to withstand corrosive environments. The exposure classification of concrete is *Very Severe* according to Table 6.4.2 (C) of AS 2159 – 2009, considering a pH less than 4 and soil Conditions A (high permeability soils). It is recommended to refer to Table 6.4.3 of AS 2159 – 2009 to select the minimum strength of concrete and concrete cover relevant for the *Very Severe* Exposure Classification and design life.
- If steel elements are used in the ground (i.e. driven H-steel piles, etc.), it is recommended adopting the Severe Exposure Classification shown in Table 6.5.2 (C) of AS 2159 2009, considering a pH less than 3 and soil Conditions A (high permeability soils). A uniform corrosion allowance of 0.04-0.1 mm/year should be adopted during the design of any underground steel structure.
- No precautions are required to be taken for timber piles. Timber is not susceptible to corrosion by acid.
- If a shallow concrete strip footing or concrete slab on grade is to be used, any organic soil such as peat and topsoil in the building footprint is to be excavated and replaced with an engineered hard fill layer of compacted aggregate in accordance with NZS 4431 or AS 3798:2007.
- The Sandy material for all lots in Stage 4-5 is consistent with a Class A site as defined by AS 2870:2011 indicating little to no ground movement from moisture changes is expected.
- The finished ground within the lots is suitable for the erection of buildings on "Good Ground" in terms of NZS 3604:2011 (Timber Framed Buildings) & NZS 4229:2013 (Concrete Masonry Buildings Not Requiring Specific Engineering Design) and related documents providing that:
 - o Standard check of excavated foundations is carried out at time of construction.
 - o Buildings subject to heavy loads or vibrations will require specific design,
 - Buildings not meeting the criteria for NZS3604:2011 and NZS4229;2013 will require specific design.



WFH Properties Limited

The Landing - Stage 4 & 5

 This report and the inferred professional opinions found within does not remove the necessity for site-specific geotechnical investigation, normal inspection and design of foundations, as would be made in natural ground. The Landing – Stage 4 & 5

10. Limitations

This report has been prepared for the benefit of WFH Properties Limited as our client & The Whangarei District Council with respect to a geotechnical completion report for Stage 4 & 5 of The Landing Subdivision, One Tree Point. It shall not be relied upon for any other purpose. The reliance by other parties on the information or opinions contained in this report shall, without our prior review and agreement in writing, be at such parties' sole risk.

Opinions and judgments expressed herein are based on our understanding and interpretation of current regulatory standards and should not be construed as legal opinions. Where opinions or judgments are to be relied on they should be independently verified with appropriate legal advice. Any recommendations, opinions, or guidance provided by Cook Costello in this report are limited to technical engineering requirements and are not made under the Financial Advisers Act 2008.

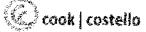
Recommendations and opinions in this report are based on data from testing undertaken on site. The nature and continuity of subsoil conditions away from the tests are inferred and it must be appreciated that actual conditions could vary considerably from the assumed model.

During excavation and construction the site should be examined by a Cook Costello Engineer or Engineering Geologist to judge whether the exposed subsoil's are compatible with the inferred conditions on which the report has been based. It is possible that the nature of the exposed subsoil's may require further investigation and the modification of the design based on this report. In any event it is essential that the firm is notified if there is any variation in subsoil conditions from those described in the report as it may affect the design parameters recommended in the report.

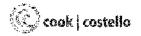
Cook Costello have performed the services for this project in accordance with the standard agreement for consulting services and current professional standards for environmental site assessment. No guarantees are either expressed or implied.

There is no investigation which is thorough enough to preclude the presence of materials at the site which presently, or in the future, may be considered hazardous. Because regulatory evaluation criteria are constantly changing, concentrations of contaminants present and considered to be acceptable now may in the future become subject to different regulatory standards which cause them to become unacceptable and require further remediation for this site to be suitable for the existing or proposed land use activities.

This report is generally appropriate for engineering use for five years, however it should be considered that future seismic events may change soil parameters and as a result, a new assessment of the site may be necessitated. Local changes in groundwater tables may also necessitate a reassessment of the site. It is recommended that if an interpretive report is more than two years old, or the proposed building that the report originally applied to have changed significantly, (e.g., layout, height, weight of building materials, foundation loads etc.) and/or design loadings have changed (e.g., design PGA levels), then the report is reviewed by the geotechnical engineer for current applicability.



The Landing Stage 4 & Stage 5 - Earthworks Completion Report



WHANGAREI DISTRICT COUNCIL Forum North ' Private Bag 9023 ' Whangarei 0148 ' New Zealand Telephone (09) 430 4200 ' 0800 WDC INFO ' 0800 932 463 ' Facsimile (09) 438 7632 Website http://www.wdc.govi.nz · E-mail mailroom@wdc.govi.nz



Statement of Professional Opinion on Suitability of Land for Building Construction Form EES-PO1

Development	The Landing -	Stage 4 and Stage 5, One Tiree Point	
Developer	WFH POPERTIES LIMITED		
Location	One Tree i	Point	
I (full name)	Stefano Rotato	ri	
of (Name and ad	ddress of firm)	Cook Costello Ltd, Norfolk House, 2 Norfolk Street, Whangarei, Northland	

Hereby confirm that

1 I am a geo-professional as defined in Section 1.2 of the WDC EES and was retained by the developer as the geoprofessional on the above development

2 The extent of my preliminary investigations are described in my Report(s) number 14333-004 dated 21/10/2021 & the conclusions and recommendations of that/those document(s) have been re-evaluated in the preparation of this report. The extent of my inspections during construction, & the results of all tests and/or re-evaluations carried out are as described in my geotechnical completion report dated 21/10/2021.

3 In my professional opinion, not to be construed as a guarantee, I consider that:

a The earth fills shown on the attached Plan No. AS EW 1D, AS EW 2D appended within Appendix 2 of the Completion Report dated 21/10/2021 have been placed in compliance with the requirements of Council & my specification

b The completed works take into account land slope & foundation stability considerations, subject to the appended foundation recommendations and earthworks restrictions, *(which should be read in conjunction with the appended final site contour plan* AS FW 1D, AS EW 2D).

- c Subject to 3(a) and 3(b) above, the original ground not affected by filling satisfies the description of 'good ground' as described in NZS3604/NZS4229
- d Subject to 3(a) & 3(b) above, the filled ground satisfies the description of 'good ground' as described in NZS3604/NZS4229
- e The original ground not affected by filling & the filled ground are not subject to erosion, subsidence, or slippage in accordance with the provisions of section 106 of the Resource Management Act 1991provided that:
 - Site-specific geotechnical investigation and design of foundations are to be carried out at the detailed design of buildings.
 - (ii) Standard inspection of excavated foundations is to be carried out at time of construction
 - (iii) Buildings subject to heavy loads or vibrations will require specific design.
- 4 This professional opinion is furnished to the TA & the developer for their purposes alone on the express condition that it will not be relied upon by any other person and does not remove the necessity for the normal inspection of foundation conditions at the time of erection of any building.
- 5 This certificate shall be read in conjunction with my geotechnical report referred to in clause 2 above & shall not be copied or reproduced except in conjunction with the full geotechnical completion report.

signature Stefan Alla

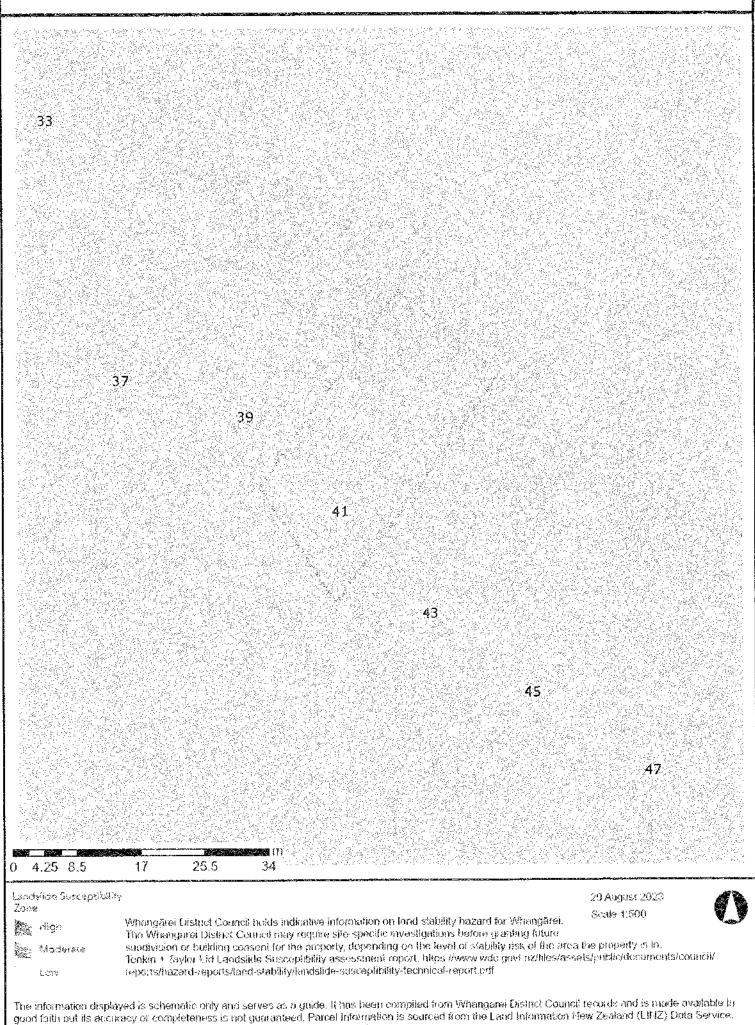
Qualifications

Date

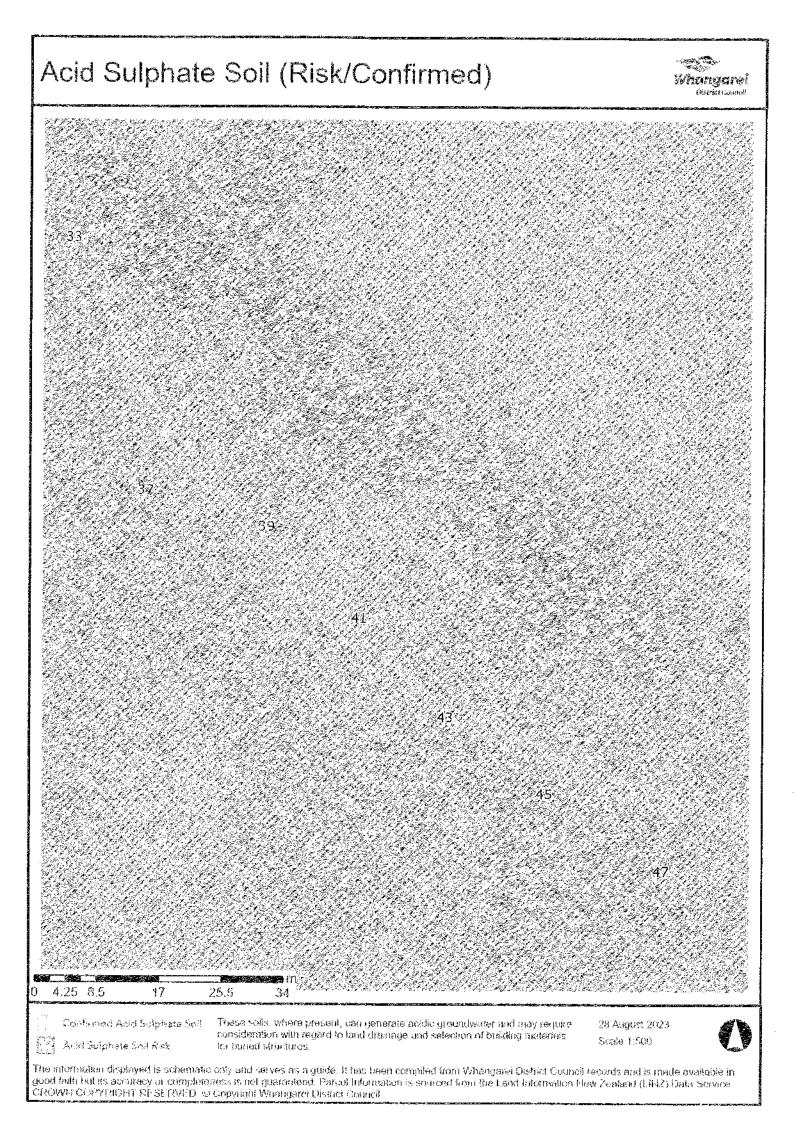
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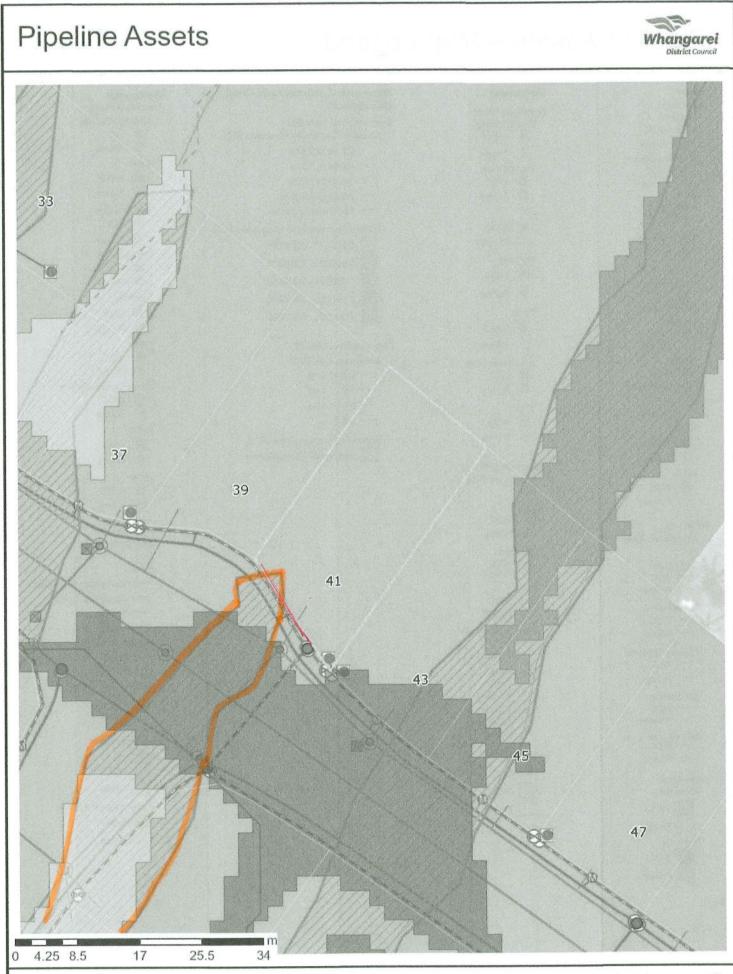
21/10/2021

Land Stability



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This information is generalized and shows the approximate location of the Public pipeline services. For digging, the As-Built engineering drawings must be used to accurately locate the services. See WDC Customer Services. 28 August 2023 Scale 1:500



The information displayed is schematic only and serves as a guide. It has been compiled from Whangarei District Council records and is made available in good faith but its accuracy or completeness is not guaranteed. Parcel Information is sourced from the Land Information New Zealand (LINZ) Data Service. CROWN COPYRIGHT RESERVED. © Copyright Whangarei District Council.

Pipeline Assets – Map Legend

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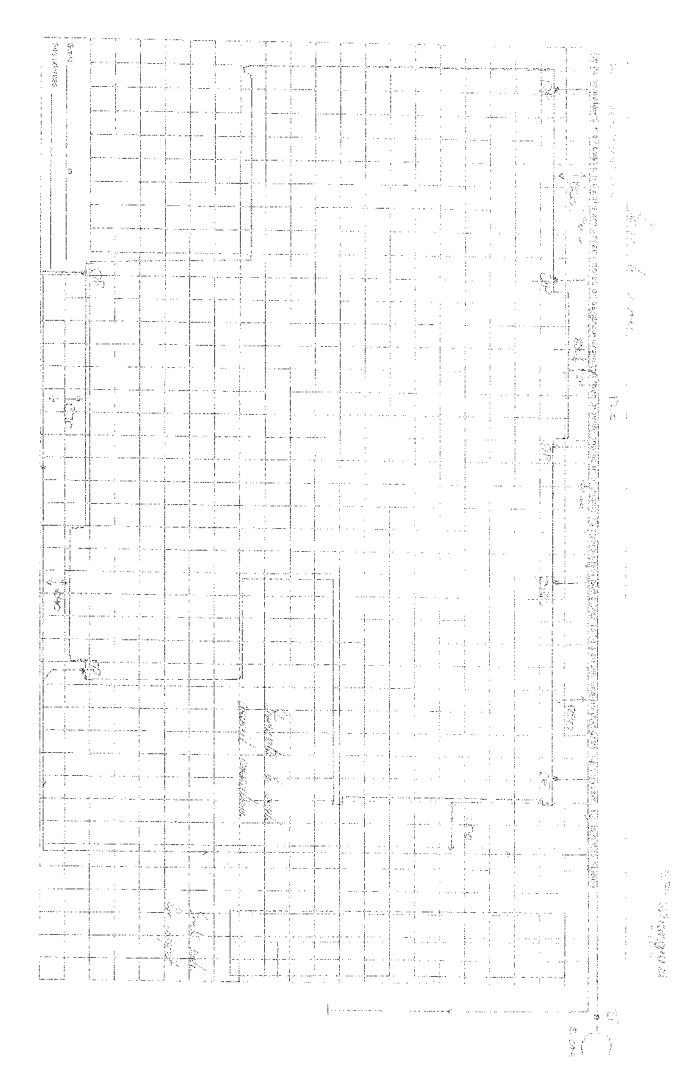
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Whangarei District Council Private Bag 9023, Te Mai Whangarei 0143 Ph:0-9-430 4200 Email: mailroom@wdc.govt.nz

Rates LIM Report

As at: Monday, 28 August, 2023

Property Number172212Legal DescriptionLOT 407 DP 564989Assessment Number0029002166Address41 Kaurinui Crescent One Tree Point 0118Record of Title(s)1009463Land Value\$380,000Capital Value\$980,000Date of Valuation01-July-2021Effective Date (used for rating purposes)01-July-2023Meter Location172212

Rates Breakdown (up to 30 June 2024)

Rates Charge	Charge Total
General Residential	\$886.39
Sewage Disposal - Residential	\$902.00
Uniform Annual General Charge	\$701.00
Regional Council Services	\$180.61
Regional Economic Development	\$8.13
Regional Emergency & Hazard Management	\$50.50
Regional Emergency Services Rate	\$11.44
Regional Flood Infrastructure	\$36.56
Regional Land and Fresh Water Management	\$106.48
Regional Pest Management	\$88.06
Regional Sporting Facilities	\$16.37
Regional Transport Rate	\$43.17
Annual Charge Total	\$3,030.71

Opening Balance as at 01/07/2023

Rates Instalments	Total
20/07/2023 Instalment	\$759.71
20/10/2023 Instalment	\$757.00
20/01/2024 instalment	\$757.00
20/04/2024 Instalment	\$757.00
Rates Total	\$3,030.71

Balance to Clear

\$2,271.00

\$0.00

N**strict** Council

Forum North, Private Bag 9023 Whangarei 0148, New Zealand 8 +64 430 4200 mailroom@wdc.govi.nz STRAN STOR BOOM AR

Form 5 **Building Consent - BC2200495** Section 51, Building Act 2004

The Building

41 Kaurinui Crescent Street address of building: Legal description of land where building is located: N/A **Building name:** N/A Location of building within site/block number: N/A Level/unit number:

The Owner

Name of owner: Contact person: Mailing address:

Street address/registered office: Phone number: Daytime: After hours: Facsimile number: Email address: Website:

One Tree Point 0118 LOT 407 DP 564989

ATK Developments Limited Anna Trainina-Knapp PO Box 35266 **Browns Bay** Auckland 0753 N/A Landline: N/A Mobile: 0212279012 N/A N/A N/A anna@atkdevelopments.co.nz N/A

First point of contact for communications with the building consent authority: Emma Thacker (A1 Homes Northland); Mailing Address: PO Box 183 Ruakaka 0151; Phone: 094330200; Email: emmathacker@a1homes.co.nz

Building Work

The following building work is authorised by this building consent: New Dwelling

This building consent is issued under section 51 of the Building Act 2004. This building consent does not relieve the owner of the building (or proposed building) of any duty or responsibility under any other Act relating to or affecting the building (or proposed building). This building consent also does not permit the construction, alteration, demolition, or removal of the building (or proposed building) if that construction,

alteration, demolition, or removal would be in breach of any other Act.

Conditions

This building consent is subject to the following conditions:

Section 90 - Inspections by Building Consent Authorities: (1) Every building consent is subject to the condition that agents authorised by the building consent authority for the purposes of this section are entitled, at all times during normal working hours or while building work is being done, to inspect(a) land on which building work is being or is proposed to be carried out; and
(b) building work that has been or is being carried out on or off the building site; and
(c) any building.

(2) The provisions (if any) that are endorsed on a building consent in relation to inspection during the carrying out of building work must be taken to include the provisions of this section.

(3) In this section, inspection means the taking of all reasonable steps to ensure that building work is being carried out in accordance with a building consent.

Construction monitoring requirements

Please contact the following consultants directly to arrange the construction monitoring identified which they have been engaged to carry out.

 Cook Castello or suitably qualified engineer: Confirm ground conditions and bearing

Copies of all site reports/records must be provided to the Building Consent Authority as work proceeds for their records. Please forward these to mailroom@wdc.govt.nz referencing the building consent number.

Inspections

The following inspections are required:

- Siting and Site Scrape Inspection
- Wastepipes
- Framing / Pre-wrap
- Half High Brick
- Preline Building
- Drainage

- Prepour
- Floor Slab
- Post Wrap / Cavity
- Preline Plumbing
- Post Line
- Final

Documents Required

Prepour

B1: Foundations - Construction monitoring records

Wastepipes

- · G12: Pipework pressure test documentation
- · G13: Underslab plumbing as-built & plumbers details

Floor Slab

B1: Slab - Construction monitoring records

Framing / Pre-wrap

B1: Truss certification

Preline Building

G12: Pipework pressure test documentation

Drainage

- · E1: Stormwater drain leakage test and as-built plans
- · G13: As-builts, drainlayer details, pipework test

Final

- LBP Memorandum For All Restricted Building Work Carried Out
- PS4 From Structural Engineer And A Copy Of All Site Notes From Inspections
- · G9: Energy works certificate

Compliance Schedule

A compliance schedule is not required for this building.

Attachments

Copies of the following documents are attached to this building consent: Information page: Now you have your Building Consent

Advice notes

Addan.

Signature: Letitia Feldon Position: Building Control Officer On behalf of: Whangarei District Council Issue Date: 10 June 2022

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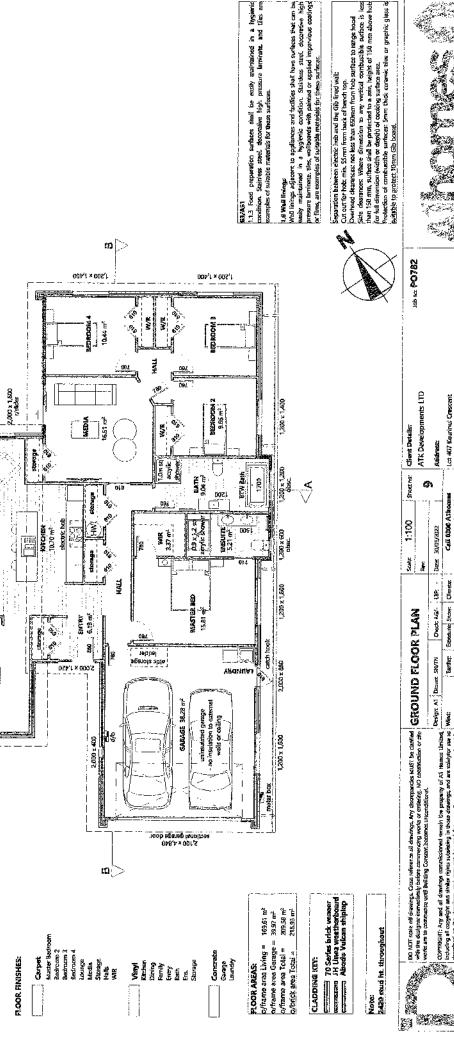
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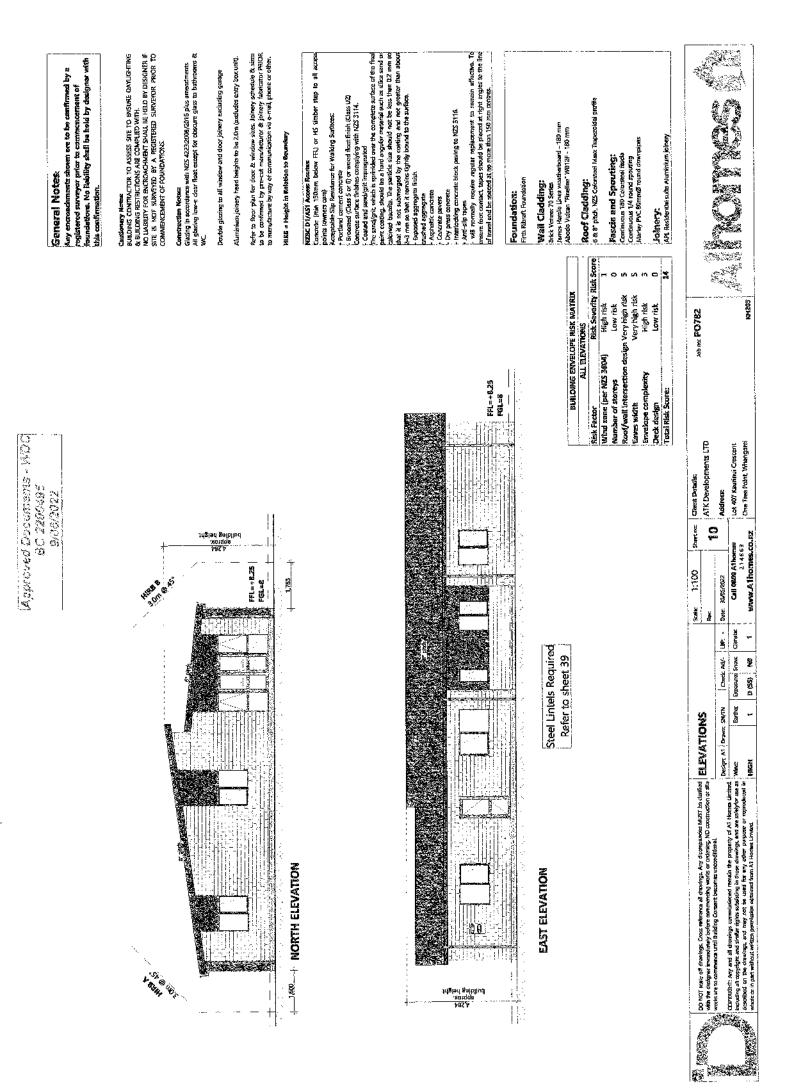
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9/08/2022

A1 Homes Northland Steve.hart@a1homes.co.nz

Dear Steve,

Ground Conditions Assessment Report for Proposed New Residential Dwelling at: Proposed Lot 407 Kaurinui Crescent, Stage 4 The Landing, One Tree Point

1.0 INTRODUCTION

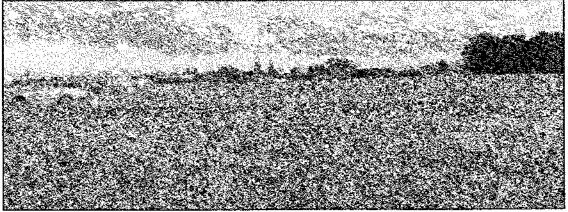
Based on your instruction, we have prepared this Ground Conditions Assessment Report regarding the central construction of a new single-level residential dwelling at the above address. The purpose of this report was to assess the ground conditions at the property for foundation design purposes in general accordance with Section 3 of NZS3604.

The subject proposed allotment does not currently have title; however, it is our understanding that the wider Stage 4 area of 'The Landing' that the proposed development is to reside within, is currently in the Resource Management Act S.223 and S.224 certification processing phase. The proposed Lot location is currently displayed on the Whangarei District Council (WDC) GIS Property Map.

2.0 SITE DESCRIPTION

The proposed subject 597m² urban property, is to be located off the northern side of Kaurinui Crescent, accessed directly north of the roundabout intersection of Kaurinui Crescent and Te Piriti Road, within Stage 4 of 'The Landing' subdivision development, in the suburb of One Tree Point.

Topographically speaking, the property and wider surrounding land is flat natured, having been formed level through an engineered cut/fill earthworks operation during the subdivision stage of the property. The 'Bulk Earthworks Plan – Finished Earthwork Contours, Stage 4 (ref: AS EW 1D)' of the Earthworks Completion Report (ECR - see section 3.0 below), indicates that the site has largely been filled up to approximate heights ranging between 1.0m to 2.0m. Contours on the plan indicate that existing ground level (EGL's) across the site range between approximately RL8.0 (southeast boundary) to RL8.4 (northwest boundary).



Site photograph of the proposed building site (northwest direction). Orange cones outline the building site.



Consulting Engineers

Tel: 021 080 12351 shaun@wjl.co.nz Job No: 115018 24 February 2022

NOCTELEND Po Box 8130 Kensington Whangarei 0145 T: F64 9 945 4188

Atticia ANO WARAPJ Po Box 11381 Cliershie Auchtaud 1542 T: +84 9 527 0195

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Consulting Engineers

3.0 PREVIOUS GEOTECHNICAL REPORT

At the time of preparing this report, we have reviewed the following Geotechnical Report, which we understand was prepared for Stages 4 and 5 of the 'The Landing' subdivision development:

• Earthworks Completion Report (ECR), One Tree Point, The Landing Stage 4 & Stage 5 (ref: 14333-004), dated 21 October 2021, by Cook Costello Ltd, for WFH Properties Ltd.

In reviewing the ECR, we note the following surmised conclusions and recommendations regarding future residential development at the proposed property:

- The test results indicate that the sites (once the peat/topsoil layer is removed) are suitable for standard foundations for buildings,
- An ultimate bearing capacity of 300kPa is available below any peat/topsoil that has been spread on-site after the completion of the earthworks. To be noted that between 100mm to 300mm of topsoil has been spread throughout the site, with an average of 200mm,
- The site is consistent with Subsoil Class C Shallow Soil sites as per NZS1170.5,
- Liquefaction potential is low. Little to no expression of liquefaction is expected within Stage 4 and Stage 5 subdivisions,
- Up to 20mm settlements could be expected under the Ultimate Limit State (ULS) earthquake case that are within the tolerancy of the New Zealand Building Code (NZBC), that is 25mm over 6m according to Clause B1/VM4 for static settlements,
- Foundation design should limit the probable maximum differential settlement over a horizontal distance of 6m to no more than 25mm serviceability limit state load combinations of AS/NZS1170 Part 0 unless the structure is specifically designed to prevent damage under a greater settlement,
- Acid Sulphate Soil Field PH Tests have been conducted. Results of lab testing suggest that soils are likely to contain very little sulphides. However,
 - It is recommended installing a waterproof membrane underneath any shallow foundations. The membrane should be extended up to the edge of the foundations to mitigate the presence of corrosive environment. If the above recommendations are followed, concrete exposure Class can be A1 according to NZS3101, and
 - No precautions are required to be taken for timber piles. Timber is not susceptible to corrosion by acid,
- If a shallow concrete strip footing or concrete slab-on-grade is to be used, any organic soll such as peat and topsoil in the building footprint, is to be excavated and replaced with an engineered hardfill layer of compacted aggregate in accordance with NZS4431 or AS3798,
- The sandy material for all Lots in Stage 4-5, is consistent with a Class A site as defined by AS2870, indicating little to no ground movement from moisture changes is expected,
- The finished ground within the Lots, is suitable for the erection of buildings on 'Good Ground' in terms of NZS3604 and NZS4229 and related documents, providing that:
 - Standard check of excavated foundations is carried out at the time of construction,
 - Buildings subject to heavy loads or vibrations, will required specific design, and
 - Buildings not meeting the criteria for NZS3604 or NZS4229, will require specific design, and

 This report and the inferred professional opinions found within, does not remove the necessity for site-specific Geotechnical investigation, normal inspection, and design of foundations, as would be made in natural ground.

NOBTHEAND Polloci 8130 Kensington Willingarai 0145 1: 464 9 945 4188

AUCKLAMD WAIKAYO Po Box 11381 Effersite Auckland 1542 11 +64 9 527 0196

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Consulting Engineers

4.0 DEVELOPMENT PROPOSAL

At the time of preparing this report, the client has provided their own set of preliminary architectural drawings (16 sheets), dated 11 February 2022 (ref: PO782). In reviewing the drawings, we note that it is proposed for a new 216m² single-level residential dwelling to be constructed centrally at the property. The drawings indicate that the dwelling is to be founded on a raft type slab, supporting lightweight framing, predominantly brick veneer cladding, and a longrun Colorsteel roof.

The architectural drawings indicate the proposed finished floor level is at RL8.25 and in assuming an approximate 0.30m thick raft slab, it is envisaged that the building platform will be founded mainly atop cut ground of less than approximately 0.50m in height.

5.0 FIELDWORK INVESTIGATION

Will carried out a walkover inspection of the property and surrounding area, and shallow ground investigation at the proposed building site, on 23 February 2022. Subsoil testing involved the following:

- The excavation of two hand auger boreholes (HA) of 75mm diameter, both to a near refusal depth of 0.80m below existing ground level (BEGL), at the south-western (HA1) and north-eastern (HA2) ends of the building site, and
- Three dynamic cone scala penetrometer Tests (DCP), were extended from existing ground level (EGL) to depths ranging between 0.60m to 0.70m BEGL, at the south-eastern, central, and north-western areas of the building site.

Both HA's encountered a 0.10m to 0.20m thick surficial topsoil layer, overlying dense engineered fill comprising of fine sand, until termination of both HA's. The fill material was consistent with the GNS Science Geology Web Map description of the property, which is noted as Karioitahi Group Stable Dune Deposits.

Groundwater was not encountered at either of our HA testing locations, which were both extended to a maximum tested depth of 1.2m BEGL. In reviewing the ECR, we note that Section 8.4.1 indicates that groundwater levels across the wider development area range between depths of 3.04m to 5.9m BEGL.

Measured blow counts per 0.10m ground penetration from below cleared ground levels (BCGL) during the three DCP's, ranged between 8 to 18 blows, to a maximum tested depth of 0.70m BEGL.

Based on the above, we conclude that 'Good Ground' soil conditions in accordance with NZS3604, underlies the proposed building site from BCGL. The fieldwork investigation data is appended to this report and the HA and DCP locations are noted on the appended site plan.

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AUCKLANG WANAYO Fo Box (138) Filershe Auckland (1542 1: +64 9 327 0196

SCHITHEPH LAKES Pa 80x 169 Wanska 9543 Ti 564 3 443 6209

CANTERNUSY T: +64 21 824 063 SITE STABILITY

6.0



Consulting Engineers

At the time of preparing this report, we note that the property is not currently mapped by the WDC GIS Hazards Map in terms of Land Instability Risk Hazard Zonation.

We consider that the risk of moderate to deep-seated slope instability impacting on the proposed development as outlined in this report, to be significantly low, based on the following:

- No obvious evidence of instability at the property and greater surrounding area,
- Flat natured topography of the property and greater surrounding area which averages 0 less than 5°,
- Encountered 'Good Ground' soil conditions in accordance with NZ\$3604, underlying the 8 building site from BCGL's,
- Absence of elevated groundwater levels at our HA testing locations and indicative depths e. noted in Section 5.0 above, and
- 8 The property has been encompassed within previous geotechnical investigations and reporting which did not identify the site as being at unsatisfactory stability risk.

Based on the above and provided the recommendations and guidance of this report are adhered to, we assess the property as a Low-Risk Land Instability Zone in accordance with the WDC definitions,

Provided the recommendations and guidance of this report are adhered to, with regard to the Building Act 2004; Sections 71-72, we believe on reasonable grounds that:

- The current proposed site development and associated building work for which an ١. application of Building Consent (BC) must be made to WDC, should not accelerate, worsen, or result in slippage or subsidence on the land on which the building work is to be carried out or any other property; and
- ij. The land beneath the building footprint and surrounding immediate amenity area is not likely to be subject to slippage and surface manifestation.

7.0 LIQUEFACTION SUSCEPTIBILITY

The noted ECR in Section 3 above, provided a detailed liquefaction assessment, ultimately concluding the following:

"Liquefaction potential is low. Little to no expression of liquefaction is expected within . Stage 4 and Stage 5 subdivisions."

Further Information and guidance may be found in the August 2020 Tonkin & Taylor Ltd Liquefaction Vulnerability Study - Whangarei District (ref: 10112149.3001.v2) and other relevant documents as specified by WDC, the NZ Building Code, and MBIE/NZGS Earthquake Geotechnical Engineering Guidance, Module 3.

NONTILAND Po Box 21(37) Kensington Whangarei 0145 7: 564 9 945 4188

ARRIAND WAIKATO 20 Aox 11551 Sherobo Auckland 1542 T: +64/9/\$227 (01:96)

SOUTHERN LAKES Po 86x 169 Wanaka \$343 7:364334436299

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Consulting Engineers

NORTH

8.0 ACID SULPHATE SOIL RISK

The noted ECR in Section 4 above, provided a detailed acid sulphate soi risk assessment, ultimately concluding the following:

- "Acid Sulphate Soil Field PH Tests have been conducted. Results of lab testing suggest that soils are likely to contain very little sulphides. However,
 - It is recommended installing a waterproof membrane underneath any shallow foundations. The membrane should be extended up to the edge of the foundations to mitigate the presence of corrosive environment. If the above recommendations are followed, concrete exposure Class can be A1 according to NZS3101, and
 - No precautions are required to be taken for timber piles. Timber is not susceptible set to corrosion by acid."

9.0 FOUNDATION / EARTHWORKS RECOMMENDATIONS

We recommend the following design and construction recommendations regarding foundations for the new dwelling and all proposed earthworks in forming the building site:

- All surficial deleterious material should be appropriately stripped from the building site. Stripping should extend a minimum horizontal distance of 1.0m beyond the edge of the raft slab foundation. A Geo-Professional inspection of the stripped/compacted subgrade should be undertaken prior to any filling works,
- All cuts should be battered back at gradients no steeper than 1V:4H as well as being appropriately dressed and planted,
- All fills up to a vertical height of 0.30m should be battered back at grades no steeper than 1V:3H. Any fills that exceed a vertical height of 0.30m and up to a vertical height of 0.60m should be appropriately laterally constrained (e.g., deepened edge beam, foundation, wall, appropriate toe protection, etc). Any proposed fills outside the imposed limits noted above, should be referred to a Chartered Geotechnical Engineer,
- Imported hardfill compacted in accordance with NZS4431 should be utilised for all fills beneath the building footprint. The compaction of hardfill should be undertaken using either a heavy plate compactor or a steel wheeled roller with low frequency dynamic compaction. Hardfill layers should not exceed 0.15m at a time,
- All exposed soils should be re-grassed and/or planted as soon as practicable to reduce the risk of erosion,
- All foundations should be designed for a geotechnical ultimate bearing capacity of 300kPa and Non-Expansive soil conditions,
- Standard NZS3604 type foundations will be suitable for isolated foundations provided they are embedded either a minimum of 0.40m below finished engineered ground levels or BCGL, and
- When finalising the development proposals, it should be checked that all foundations lie outside 45° envelopes rising from 0.50m below the invert of service trenches and in accordance with the WDC requirements (policy #0022).

NORTHLAND Po Lox 8120 Kensington Whangarei 0145 T: +64 9 945 4188

AUCSLAND WAIKATO Po Dox 11381 Effershe AucHand 1542 F: +64 9 527 0195

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Should you have any further queries concerning the above, please contact me directly.

Yours faithfully, Wilton Joubert Ltd.

S. Page Engineering Technician

REPORT ATTACHMENTS

- Site Plan (1 sheet),
- HA Logs (2 sheets), and
- DCP Logs (1 sheet)

D. Soric BE, CPEng, CMEngNZ

Consulling Engineers

MANTH AND Po Box 5130 Kensington Whangarei 0145 T: -64 0 945 4188

ABCKLAND WARATO Po Box 11381 Effective Aucklaarl 1542 T: *64 9 527 0196

SOUTHERN LAKES Po Box 159 Wanaka 9345 11 - 64 3 443 6209

CANTERBLEY T: +64 21 824 063



Form 7 **Code Compliance Certificate**

Section 95, Building Act 2004

The Building

Street address of building: Legal description of land where building is located: Building name: Location of building within site/block number: Level/unit number: Current, lawfully established, use:

Year first constructed:

The Owner

ATK Developments Limited Name of owner: Anna Trainina-Knapp Contact person: Mailing address: Street address/registered office: N/A Landline: N/A Mobile: 0212279012 Phone number: Landline: N/A Mobile: 0212279012 Daytime: Landline: N/A Mobile: 0212279012 After hours: N/A Facsimile number: anna@atkdevelopments.co.nz Email address: First point of contact for communications with the council/building consent authority: Emma Thacker (A1 Homes Northland); Mailing Address: PO Box 183 Ruakaka 0151; Phone: 094330200; Email: emmathacker@a1homes.co.nz

Building Work

Building consent number: **Description:** Issued by:

BC2200495 New Dwelling Whangarei District Council

Code Compliance

The building consent authority named below is satisfied, on reasonable grounds, that the building work complies with the building consent.

Signature: Mike Broeshart Position: Building Control Officer - Snr - Inspections Forum North, Private Bag 9023 Whangarei 0148, New Zealand F +64 430 4200 8 mailmom@wdc.govt.nz WARE WITH BEFORE

41 Kaurinui Crescent, One Tree Point 0118
LOT 407 DP 564989
N/A
N/A
N/A
2.0 Housing: 2.0.2 Detached Dwelling with 4
occupants
2022

PO Box 35266, Browns Bay, Auckland

On behalf of: Whangarei District Council Date: 16 January 2023



FORUM NORTH PRIVATE BAG 9023, WHANGAREI, NEW ZEALAND TELEPHONE 09 430 4200 FAX 09 438 7632

VEHICLE CROSSING PERMIT APPLICATION NO: VC220104 Whangarei District Council Public Places ByLaw Received: 19 May 2022 Issued: 02 June 2022

Algonierun

ATK Developments Limited PO Box 35266 Browns Bay Auckland 0753

Agenti

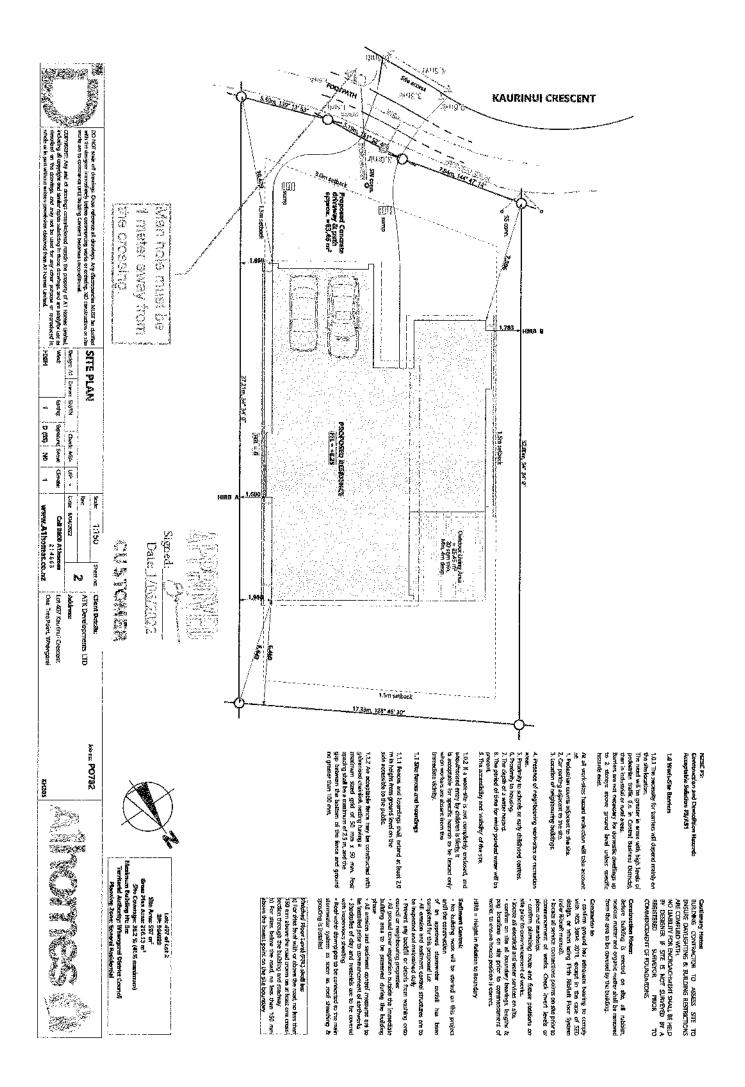
A1 Homes Northland

Property ID: 172212 Street Address: 41 Kaurinui Crescent One Tree Point 0118 Legal Description: LOT 407 DP 564989

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THIS IS A VEHICLE CROSSING PERMIT APPLICATION ONLY

COUNCIL'S TOTAL CHARGES FOR THIS VEHICLE CROSSING PERMIT ARE: \$428.00 PAYMENTS RECEIVED TO DATE: 02/06/2022 AMOUNT: \$428.00





In reply please quote PU221209 Or ask for PU221209 - Appointment

15 June 2022

A1 Homes Northland P O Box 35-266 Browns Bay Auckland 0753

Dear Sir/Madam

Public Utility Service Application No. PU221209

Site Address: 41 kaurinul Credscent, One Tree Point -- Lot 407

The application at the above address has been approved as at 15/06/2022.

Conditions:

- The work shall be undertaken by **certified drainlayer approved to install and commission the pressure sewer product** by an approved pressure sewer product supplier.
- The pumping unit to be installed shall be an approved pressure sewer product suplied by an approved supplier. This includes the tank, pump and the alarm panel.
- The pumping unit shall be commissioned by the technology supplier or their agent.
- Installation shall be in accordance with the Environmental Engineering Standards
- Consent from the Waste and Drainage Engineering Officer is required to use compression couplings.

The following information shall be supplied to Council within one month after the commissioning of the new pressure sewer pumping unit.

- Final As-built plans. The as built plan will have to comply with the Council's Environmental Engineering Standards.
- Technology supplier's commissioning checklist.
- Proof of warranty for pump equipment (minimum 24 months warranty period).
- Information for installation testing e.g pump chamber waterproof test.

The applicant must arrange for the Waste and Drainage Engineering Officer to carry out an inspection prior to backfilling the new drainage and pump unit a minimum of 48 hours ahead of time required. To arrange for an inspection please contact us on Council's main number 09 430 4200 to ensure that the request reaches the appropriate person.

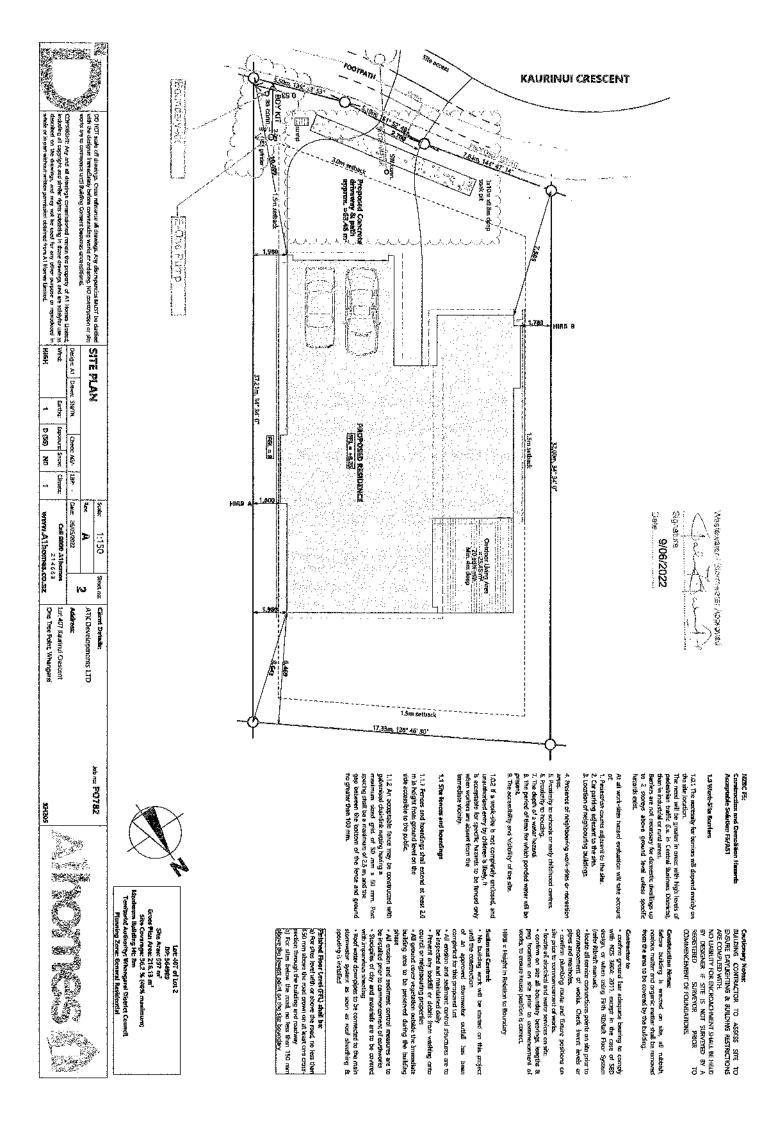
Final sign off for the Public Utility consent will occur after the passing of Council's inspection, and provision of approved as built plans provided that:

Engineering Standards and any Special Conditions are complied with as noted above.

Please keep this letter and the approved plans on site, as the contractor and the inspector will require verification of Council's approval. Thank you.

Yours faithfully

Infrastructure and Services Support Assistant





Forum North, Private Beg 9023 Whengarel 0348, New Zealand 7 +649 430 4200 9 Inailrosin@wdc.govt.nz size280.(c.govt.s2

15 June 2022

A1 Homes Northland P O Box 35-266 Browns Bay Auckland 0753

Dear Sir/Madam

Utility Services Application No. PU221209

Site Address: 41 Kaurinui Crescent One Tree Point 0118 - Lot 407

The application at the above address has been approved as at 15/06/2022.

Condition:

Council approved contractor to;

Relocate water service connection

as per Water Services approved drawings in accordance with manufacturer's instructions and WDC Bylaws and Standards.

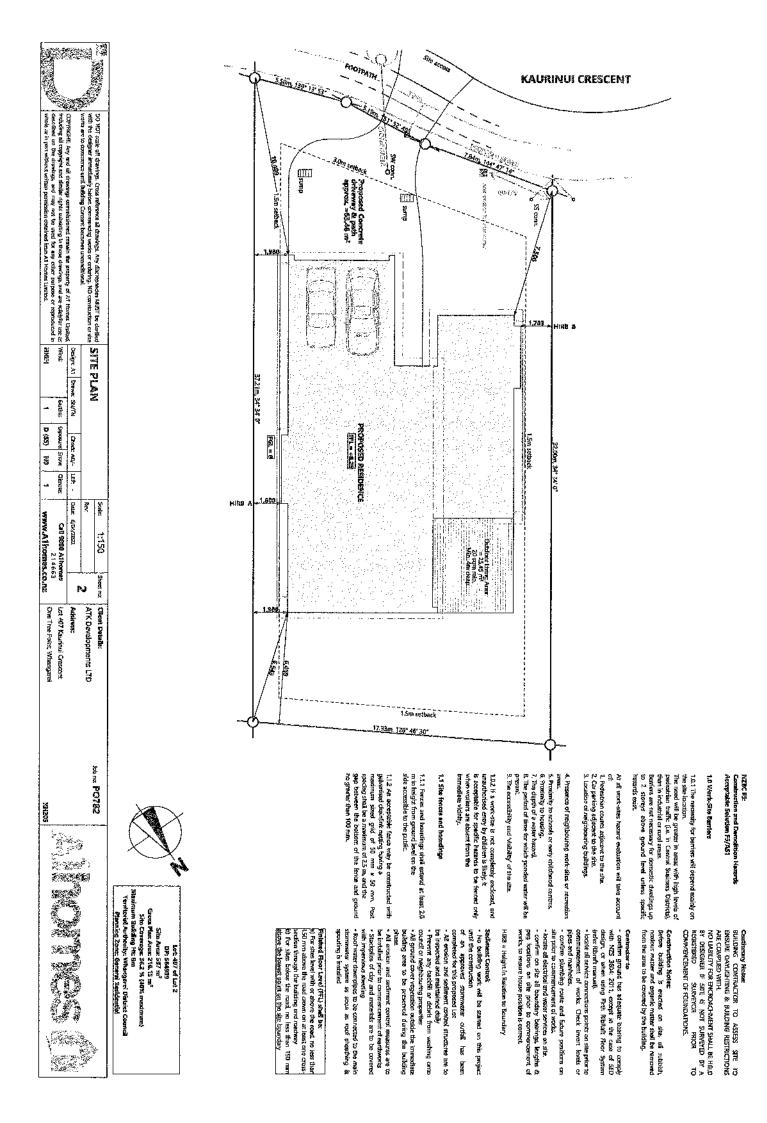
Please have this letter and the approved plans on site at time of inspection, as the inspector will require verification of Council's approval.

To arrange for an inspection please contact the infrastructure Projects and Support Administration Team on (09) 470 3137. Requests for inspections requiring WDC presence are to be made a minimum of 24hrs ahead of time. The contractor may complete the connection and backfill if the inspection was booked but not attended by the inspector at the time requested.

It is the applicant's responsibility to ensure the final 'as-built' drawing is supplied within one month of inspection. The as-built will have to comply with the Council's current Environmental Engineering Standards.

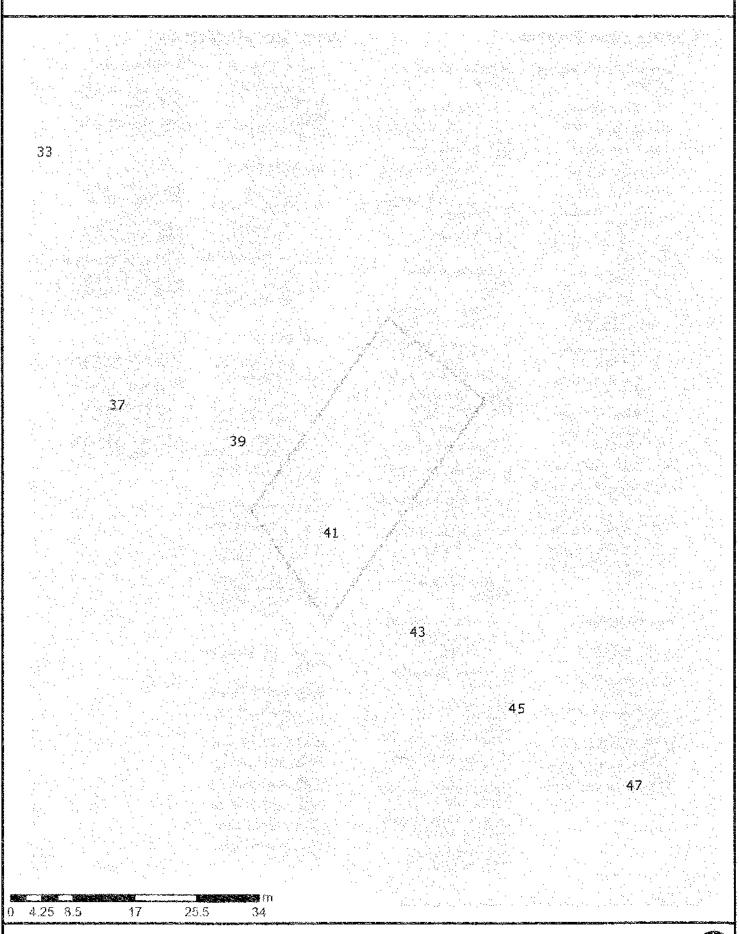
Yours faithfully

Infrastructure Support



Operative District Plan - Area Specific Matters





The information displayed is schematic only and serves as a gride, it has been compiled from Whangaret District Council 2: records and is made available in good fails but its accuracy or completeness is not guaranteed.

2S'August 2023 Scale 1 900

Parcel Internation is sourced from the Land Information New Zealand (UNZ) Data Survice CROWD COPYRIGHT RESERVED IN Copyright What gare fushed Council.

Operative District Plan – Map Legend



Istrict Wide Matters				Area	Area Specific Matters		
	infrastructure and		rical and Gultural		Multi Title Sko	industrial Zones	
Transpo		Value			Designation	Light Industria	
	rport Runway	*	Notable Tree		Precinct Development Area	Heavy Indust	
	licative Road	jan ser	Heritage Item Overlay	<u> </u>	Development Area	Zone Zone	
	itional Road		Heritage Area	Port	lential Zones		
	igional Road	\$	Sites of Significance for Maori	Neon	251(818) & VIG N	Own Press and	
	terial Road		Areas of Significance	:	Large Lot Residential Zone	Open Space and Recreation Zones	
	imary Collector pad	4000 Star (19)	Areas of Significance to Maori Papakáinga		Low Density Residential Zone	Natural Open Space Zone	
Se Se	condary Collector ad	Same and	T apononiga				
	au cess Rnad	Natur	al Environment		General Residensal Zone	Dpen Space .	
	-,,	Value			Medium Density Residential Zono	Sport and Act	
	w Volume Road		Esplanade Priority	8.941.8	Residential Zono		
Sh Pri	rategic Road otection Area		Area	Rural	Zones		
Sh	rategic Railway otection Line	Lon Maria	Coastal Marine Area (CMA) boundary			Special Purpose Zi	
Re	scue Helicopter	11 12 1.20	Goat Control Areas		Settlement Zone Residential Sub- Zone	Airport Zone	
	ght Path		QRA Quarrying Resource Area		Settlement Zone	Hospital Zone	
3 Na	itional Grid Tower		QRA Mining		Centre Sub-Zone	Port Zone	
	nthpewer Tower SL-Cat1	<u> 288</u>	QRA Buffer		Settlement Zone Industry Sub-Zone	Ruakaka Equ	
<u></u> Na	ltional Grid Line	in a se	QRA 500m Indicative	erest.		and the Promo	
* 3 see 1 No	nthpower Overhead ilical Line Cel-Cat1		Setback Outstanding Natural				
No.	Northpower Critical		Outstanding Natural Feature	1999 av	Future Urban Zone		
Ov	erhead Lines CEL		Outstanding Natural Landscape	17,2,72 			
≈∝∝ ปก	rthpower Critical Iderground Lines		Landscapts	,4457 (A.	Sirategic Rural Industries Zone		
CE	-1.	G <i>e</i> nei Matte	ral District Wide rs		Fonterra Kauri Milk Processing SRIZ - Ancillary Irrigation		
Hazards	and Risks	*****	Air Noise Boundary		Fams		
Co Ha	eastal Erosion Izant 1	and some	Outer Control Boundary	_	nercial and Mixed		
	astal Erosion izord 2		Helicopter Hovering Area	Zone	s Local Centre Zone		
122 File	od Susceptible eas	and and and a second	Nolse Control Boundary Overlay	100 1 100 12	Neighbaurhood Centre Zone		
	ning Hazard Area t	\mathbb{Z}	Rail noise alert area	10000	Commercial Zone		
277] MI	ning Hazard Area 2		Rail vibration alert area		Mixed Use Zone		
SSI M	ning Hazard Area 3		Coastal Environment Overlay		Town Centre Zone		
			Outstanding Natural Character Area		City Centre Zone Waterfront Zone		
		the states of the second	Character area	(1) (1) (1) (1) (1) (1) (1) (1) (1) (1)			

The information displayed is schematic only and serves as a guide.

It has been compiled from Whangarei District Council records and is made available in good faith but its accuracy or completeness is not guaranteed. Parcel Information is sourced from the Land Information New Zeatand (LINZ) Data Service.

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Original Sheet Size 210x297mm